OWNER

North Ogden City
505 East 2600 North
North Ogden, UT 84414
(801) 782-7211
(801) 737-0587

ARCHITECT

Method Studio, Inc.
360 W. Aspen Ave.
Salt Lake City, UT 84101
(801) 532-4422

CIVIL

Ensign Engineering and Land Surveying
45 West 10000 South, Suite 500
Sandy, UT 84070
(801) 255-0529

STRUCTURAL

BHB Structural Engineers
2766 South Main Street
Salt Lake City, UT 84115
(801) 355-5950

MECHANICAL

Spectrum Engineers
324 South State Street, Suite 400
Salt Lake City, UT 84101
(801) 328-5151

ELECTRICAL / THEATHER

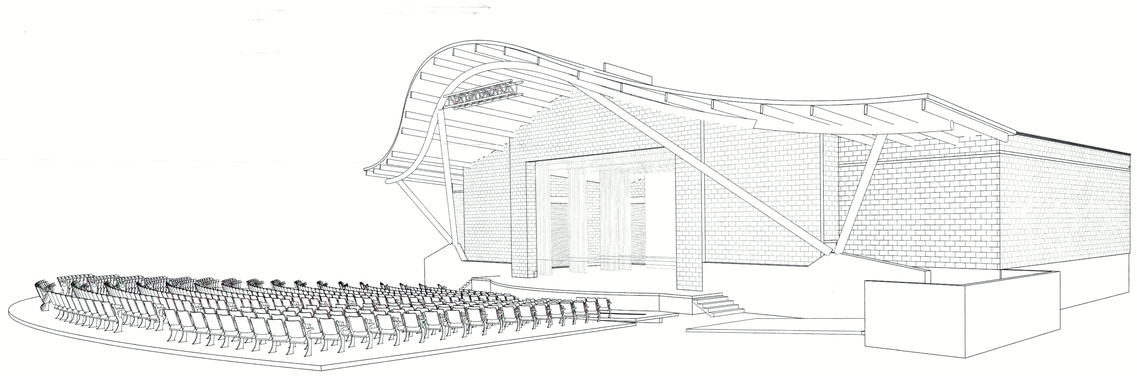
Spectrum Engineers
324 South State Street, Suite 400
Salt Lake City, UT 84101
(801) 328-5151

LANDSCAPE

Loft Six Four
9500 South 500 West
Sandy, UT 84070
(801) 580-3325

CONTRACTOR

Wadman Corporation
2920 South 925 West
Ogden, UT 84401
(801) 621-4185



North Ogden Amphitheater

Phase 1 Bid Package 2 12-1-17

LICENSED ARCHITECT
5610739-0301
KELLY BOYD MORGAN
STATE OF UTAH

12-1-2017

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360 west aspen avenue
salt lake city, utah 84101
phone:(801) 532-4422

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

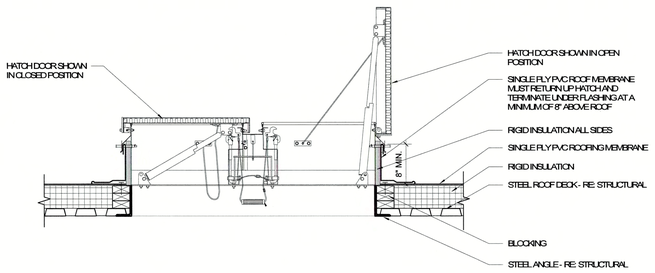
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date: 12-1-17

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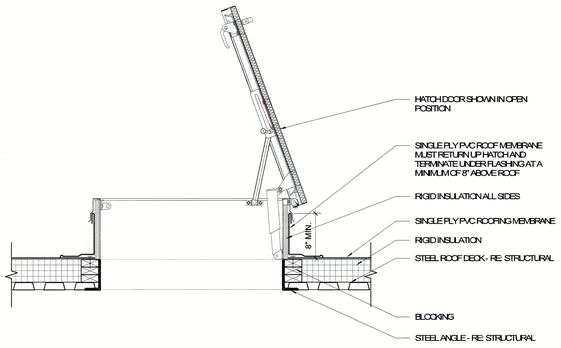
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Cover Sheet

sheet:
G001

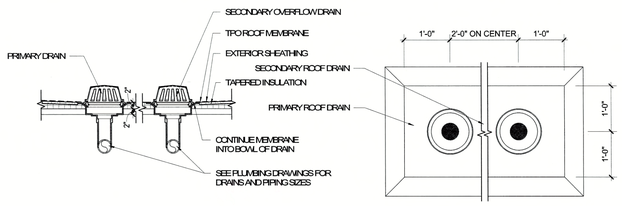
Phase 1 Bid Package 2



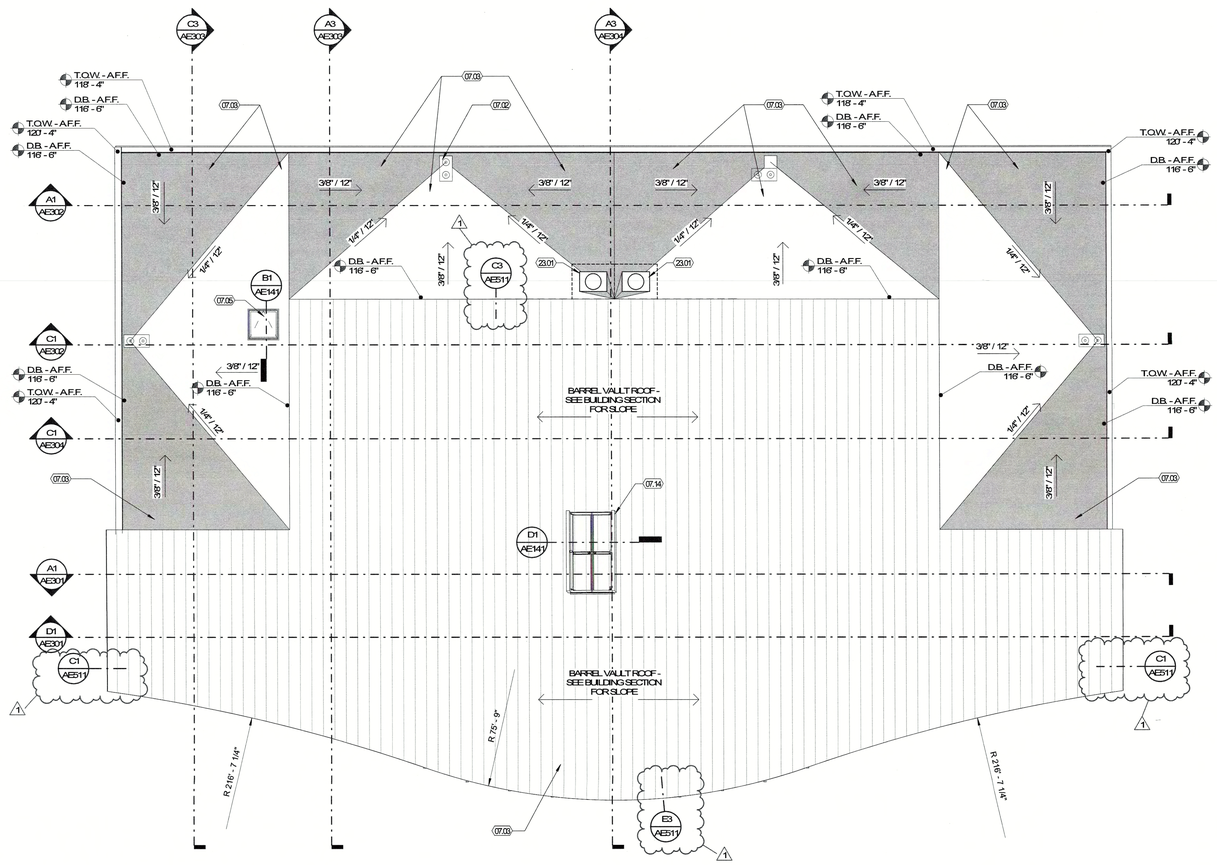
D1 Roof Smoke Vent Hatch
1" = 1'-0"



B1 Roof Access Hatch
1" = 1'-0"



A1 Roof Drain Detail
3/4" = 1'-0"



A3 Amphitheater Roof Plan
1/8" = 1'-0"

GENERAL NOTES - ROOF PLAN

1. GENERAL CONTRACTOR SHALL FIELD VERIFY ALL CONDITIONS AND SHALL REPORT TO THE ARCHITECT ANY UNKNOWN CONDITIONS, ERRORS, OR CONFLICTS IN THE DRAWINGS BEFORE BEGINNING WORK
2. DO NOT SCALE DRAWINGS
3. CRICKET AT ALL MECHANICAL ITEMS, SKYLIGHTS, ROOF HATCHES; 1/4" PER FOOT BACKSLOPE MINIMUM
4. SEE BUILDING AND WALL SECTIONS FOR ADDITIONAL ROOF DETAILS; SEE SHEET SERIES AE500 FOR ROOF ASSEMBLY CONSTRUCTION
5. ROOF/OVERFLOW DRAIN LOCATIONS SHOWN ARE APPROXIMATE; EXACT LOCATIONS SHALL BE DETERMINED IN FIELD. ROOF/OVERFLOW DRAINS TO BE PLACED TO ONE SIDE OF STRUCTURAL ELEMENT (IF EXISTS) AND INSTALLED AT LOWEST POINT
6. ALL GIVEN DECK BEARING POINTS ARE AT GRID LINE LOCATIONS UNLESS NOTED OTHERWISE

KEYED NOTES

07.02 ROOF & OVERFLOW DRAIN.
07.03 SINGLE PLY ROOFING MEMBRANE
07.05 ROOF ACCESS HATCH
07.14 AUTOMATIC SMOKE VENT
23.01 MECHANICAL EQUIPMENT, RE: MECH PLANS.

LICENSED ARCHITECT
5610739-0301
KELLY BOYD MORGAN
STATE OF UTAH

12-1-2017

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

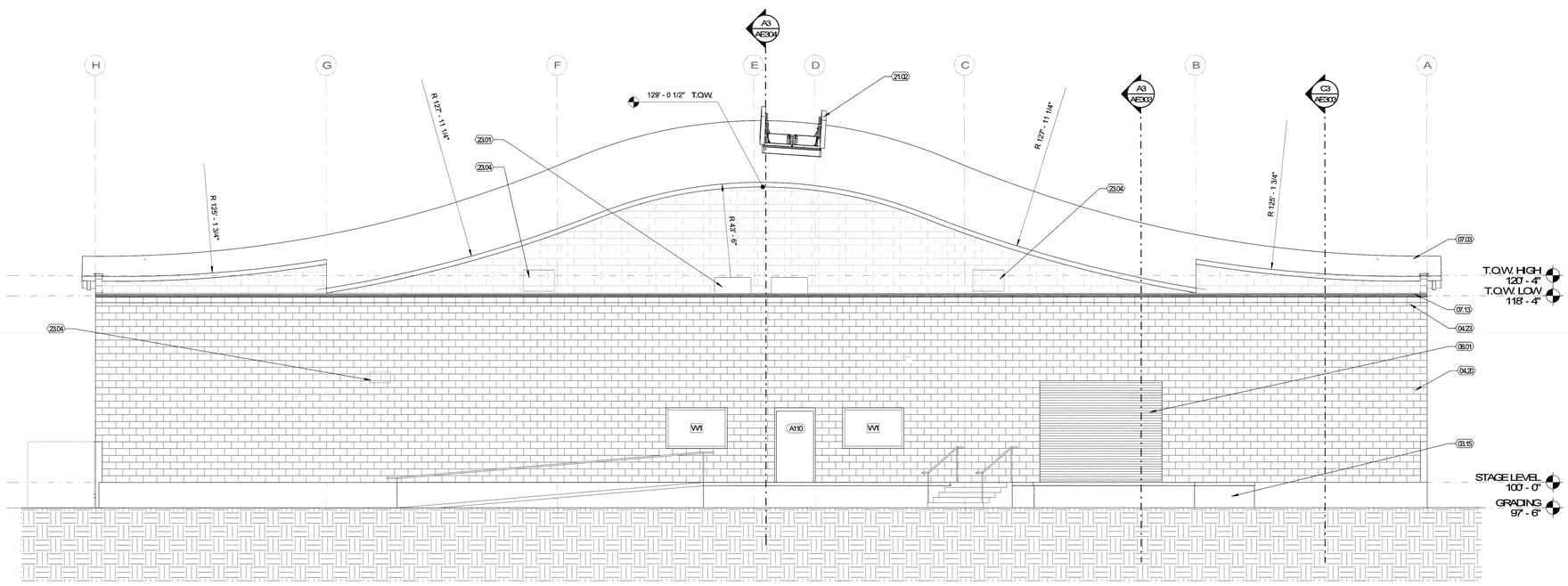
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date: 12-1-17

revisions:
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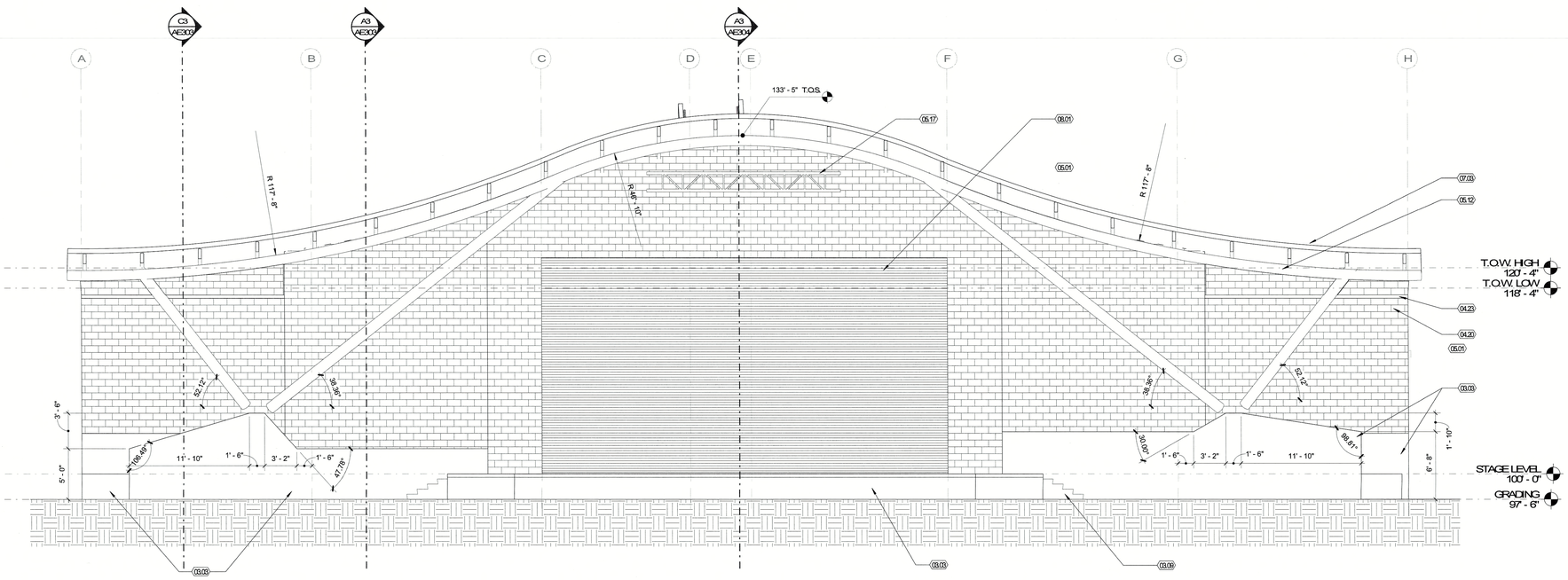
title:
Roof Plan Amphitheater

sheet:
AE141

Phase 1 Bid Package 2



C1 North Exterior Elevation
3/16" = 1'-0"



A1 South Exterior Elevation
3/16" = 1'-0"

KEYED NOTES

03.03 ARCHITECTURAL CONCRETE
03.09 CAST-IN-PLACE CONCRETE STAIR TREADS
03.15 CONCRETE LOADING DOCK
04.20 HONED CMU WALL
04.23 4" RECESSED CMU BAND
05.12 STEEL PIPE TRUSS, RE: STRUCTURAL
05.17 PREFAB. SUSPENDED LIGHT TRUSS, RE: THEATRICAL DRAWINGS
07.03 SINGLE PLY ROOFING MEMBRANE
07.13 CONT. PREFINISHED METAL PARAPET CAP
08.01 OVERHEAD ROLLING DOOR
21.02 ROOF SMOKE HATCH
23.01 MECHANICAL EQUIPMENT, RE: MECH PLANS.
23.04 LOUVER, RE: MECH.

LICENSED ARCHITECT
5610739-0301
KELLY BOYD MORGAN
STATE OF UTAH
12-1-2017

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

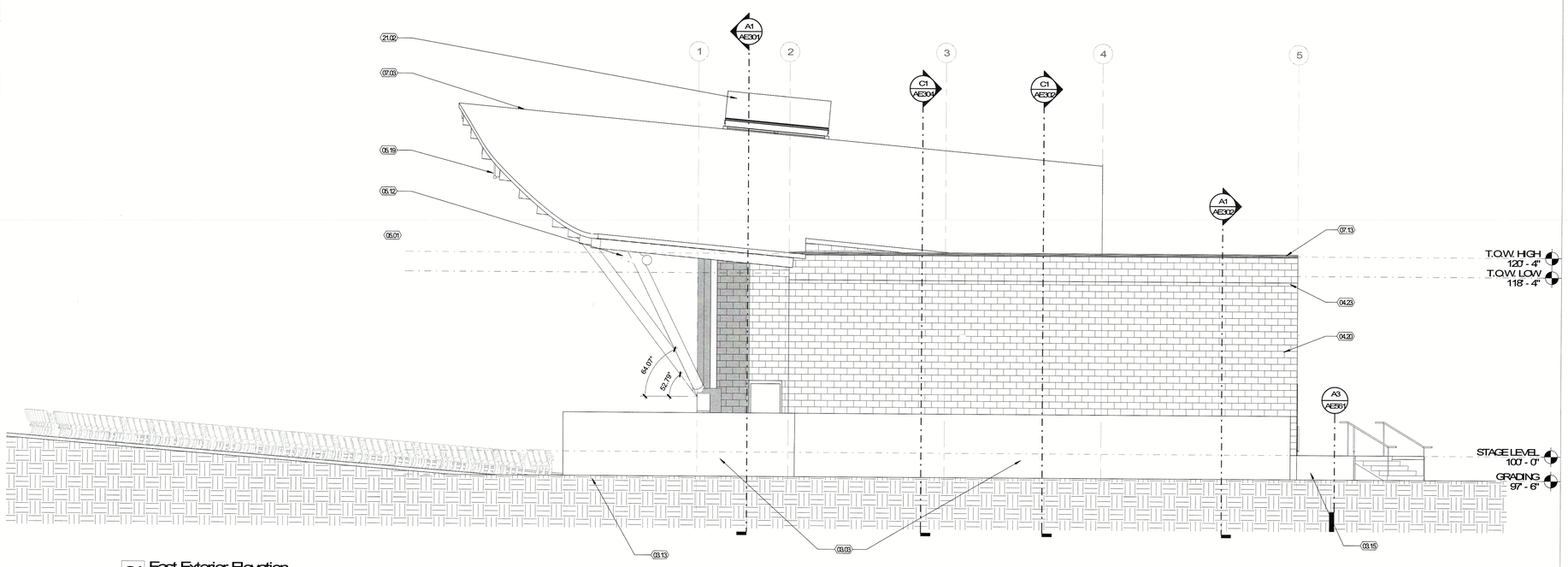
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date: 12-1-17

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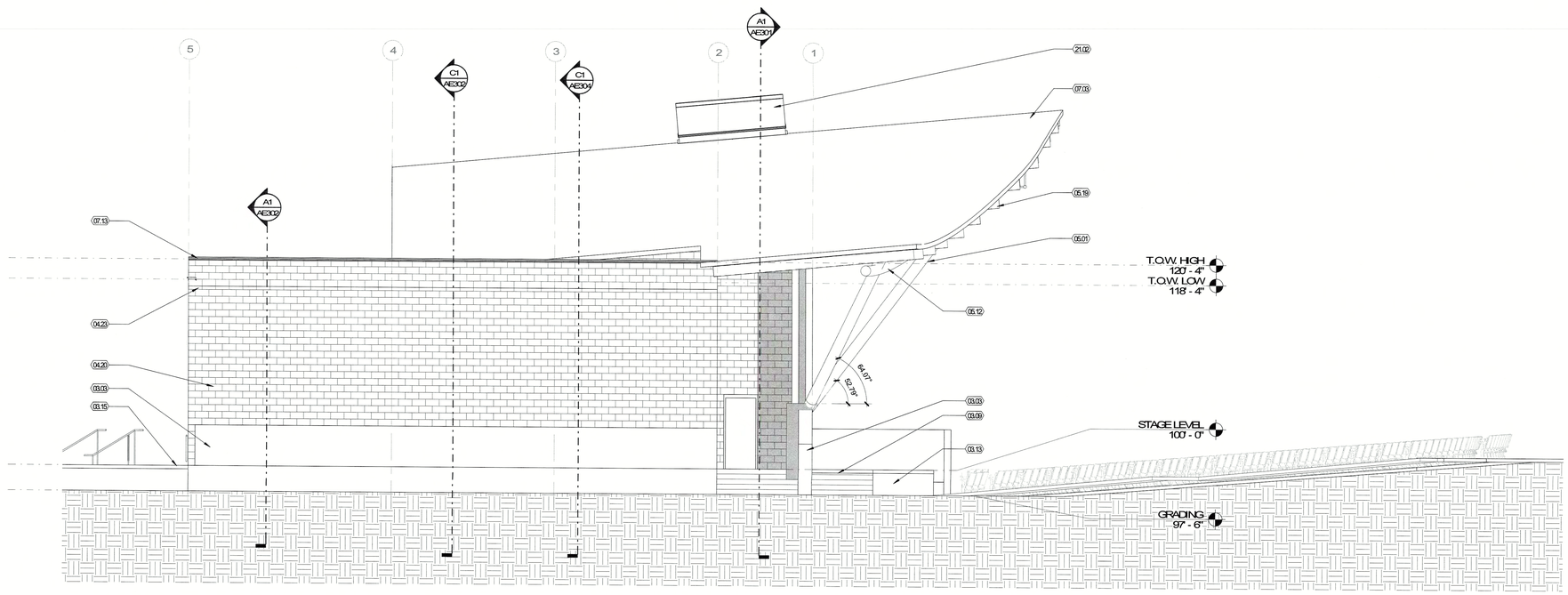
title:
Exterior Elevations Amphitheater

sheet:
AE201

Phase 1 Bid Package 2



C1 East Exterior Elevation
3/16" = 1'-0"



A1 West Exterior Elevation
3/16" = 1'-0"

KEYED NOTES

03.03 ARCHITECTURAL CONCRETE
03.09 CAST-IN-PLACE CONCRETE STAIR TREADS
03.13 CONCRETE LANDING
03.15 CONCRETE LOADING DOCK
04.20 HONED CMU WALL
04.23 4" RECESSED CMU BAND
05.01 STEEL COLUMN, RE: STRUCTURAL
05.12 STEEL PIPE TRUSS, RE: STRUCTURAL
05.19 TUBE STEEL BEAM, RE: STRUCTURAL
07.03 SINGLE PLY ROOFING MEMBRANE
07.13 CONT. PREFINISHED METAL PARAPET CAP
21.02 ROOF SMOKE HATCH

LICENSED ARCHITECT
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MORGAN
STATE OF UTAH

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

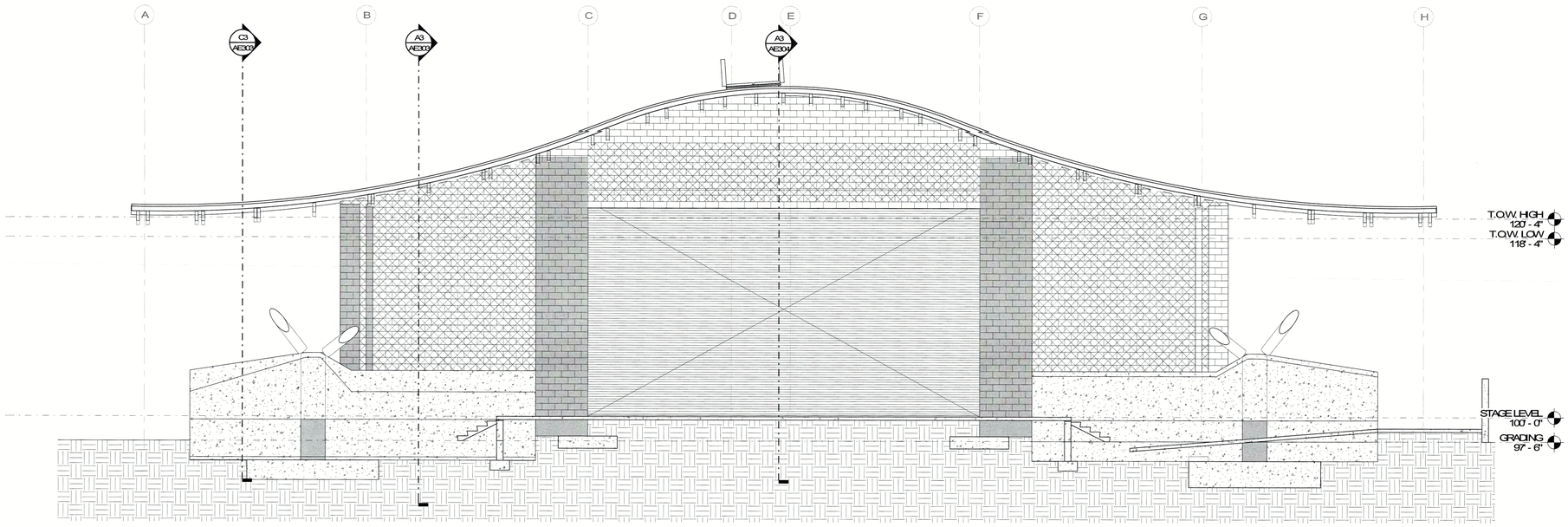
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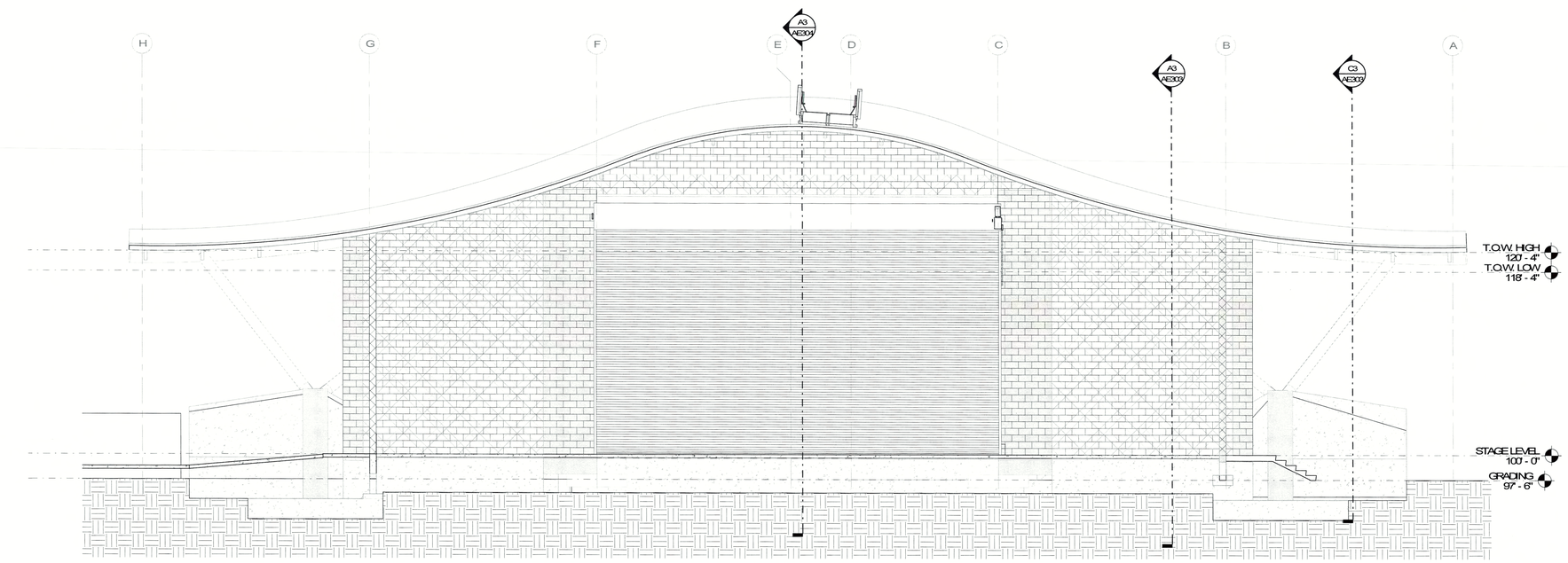
title:
Exterior Elevations Amphitheater

sheet:
AE202

Phase 1 Bid Package 2



D1 Building Section North 1
3/16" = 1'-0"



A1 Building Section North 2
3/16" = 1'-0"

LICENSED ARCHITECT
9510739-0301
KELLY BOYD MORGAN
STATE OF UTAH

12-1-2017

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

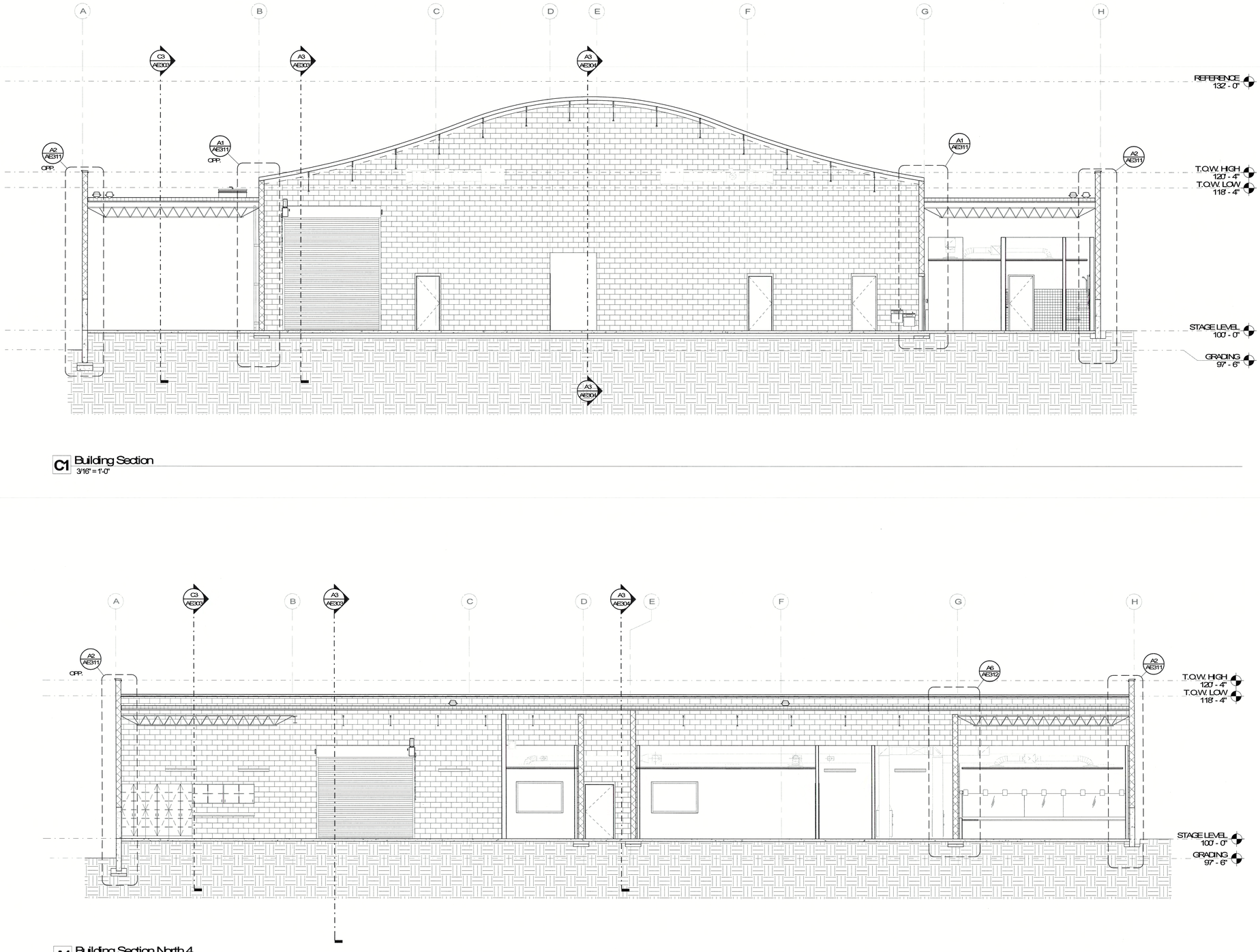
project#: 17.0555
date: 12-1-17

revisions:

title:
Building Sections

sheet:
AE301

Phase 1 Bid Package 2



C1 Building Section
3/16" = 1'-0"

A1 Building Section North 4
3/16" = 1'-0"

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5610739-0301
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STATE OF UTAH

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project:
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2375 Fruitland Dr, North Ogden, UT 84414

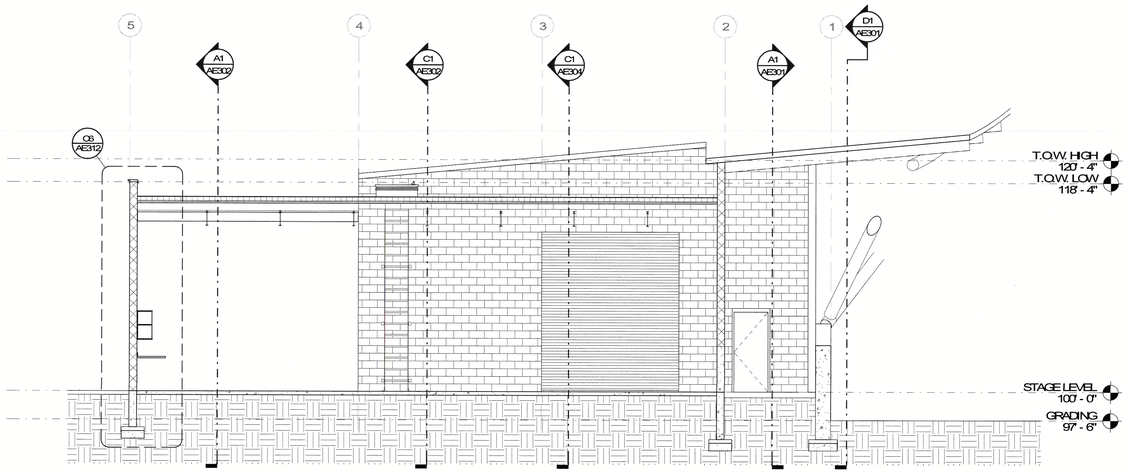
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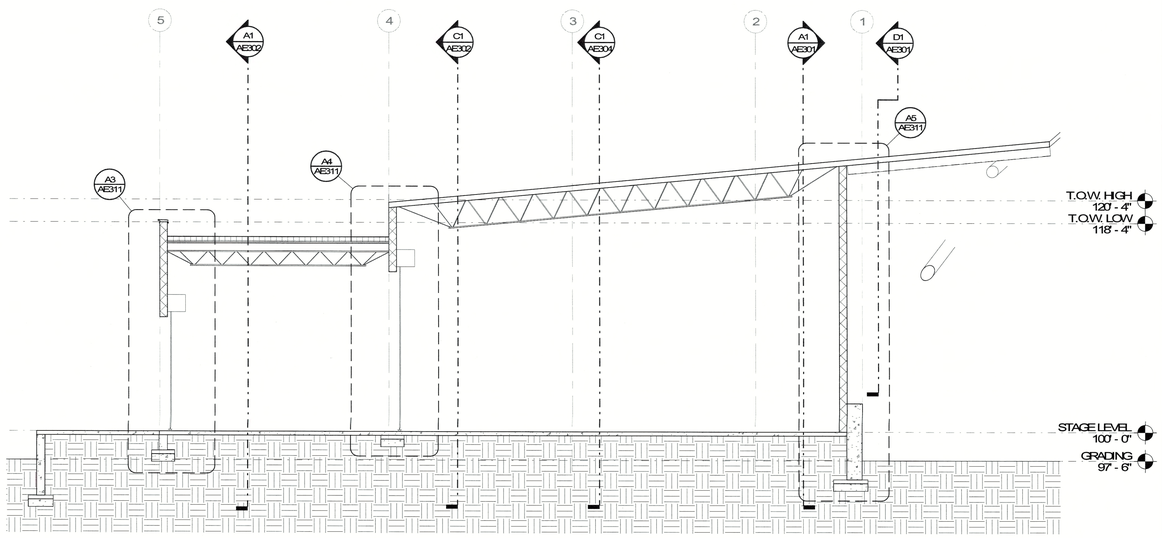
title:
Building Sections

sheet:
AE302

Phase 1 Bid Package 2



C3 Building Section East 1
3/16" = 1'-0"



A3 Building Section East 2
3/16" = 1'-0"

LICENSED ARCHITECT
5610739-0301
KELLY BOYD MORGAN
STATE OF UTAH

12-1-2017

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project:
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2375 Fruitland Dr, North Ogden, UT 84414

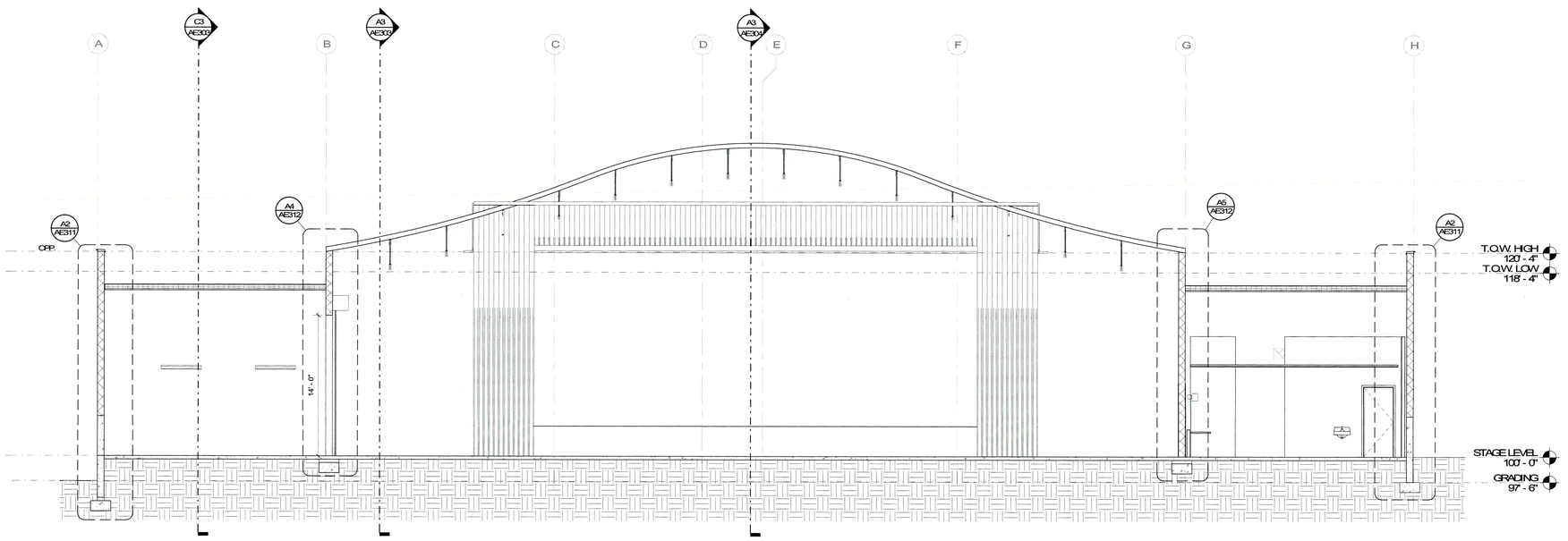
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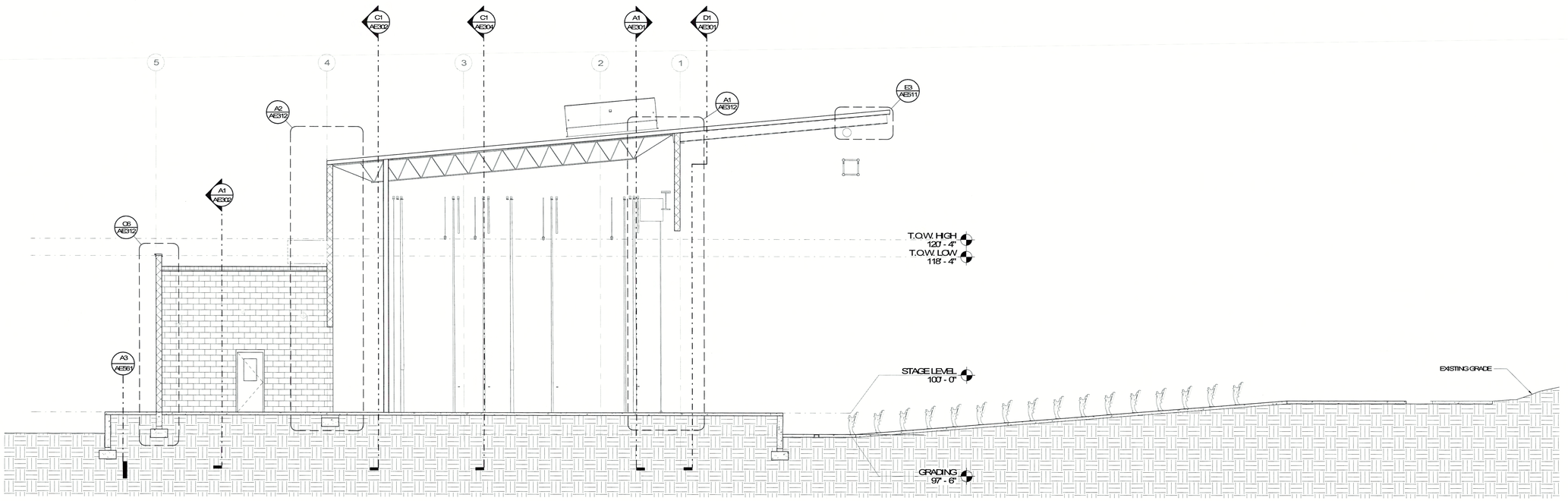
title:
Building Sections

sheet:
AE303

Phase 1 Bid Package 2



C1 Building Section North 5
3/16" = 1'-0"



A3 Building Section
3/16" = 1'-0"

LICENSED ARCHITECT
5810739-0301
KELLY BOYD
MORGAN
STATE OF UTAH

12-1-2017

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

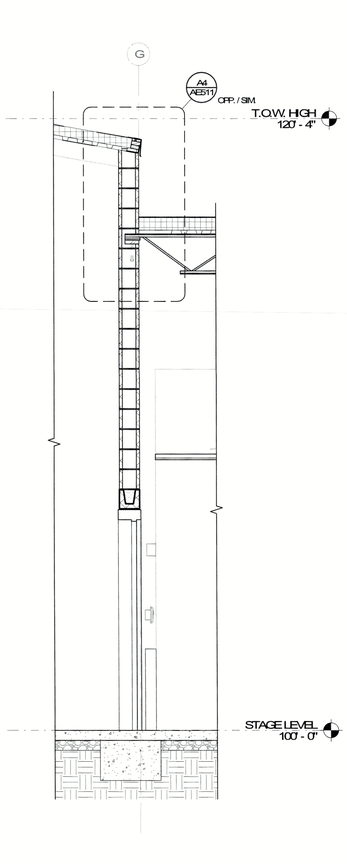
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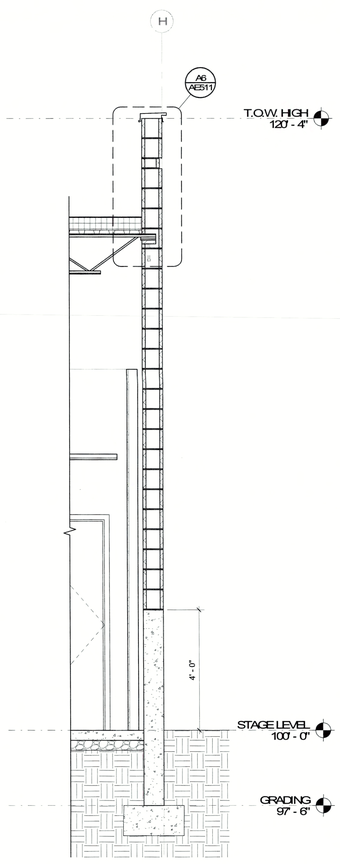
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sheet:
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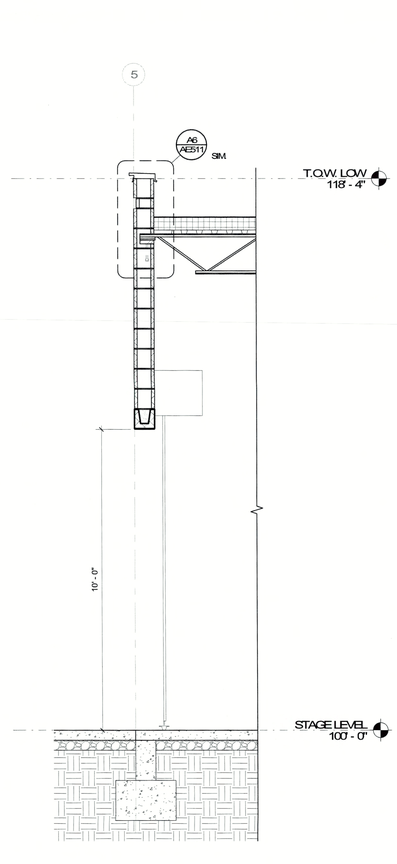
Phase 1 Bid Package 2



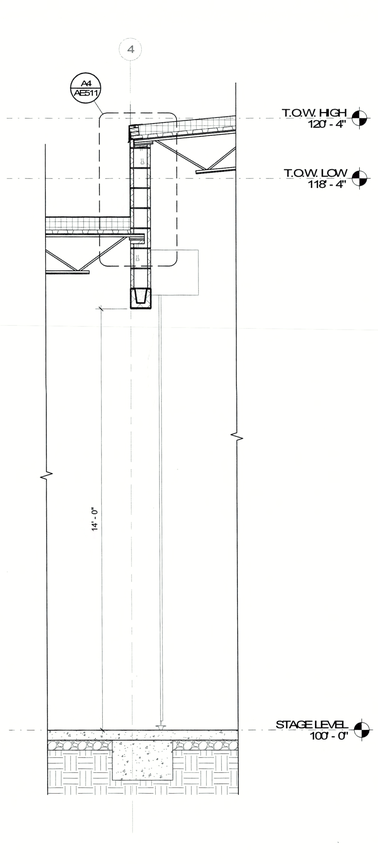
A1 WALL SECTION
1/2" = 1'-0"



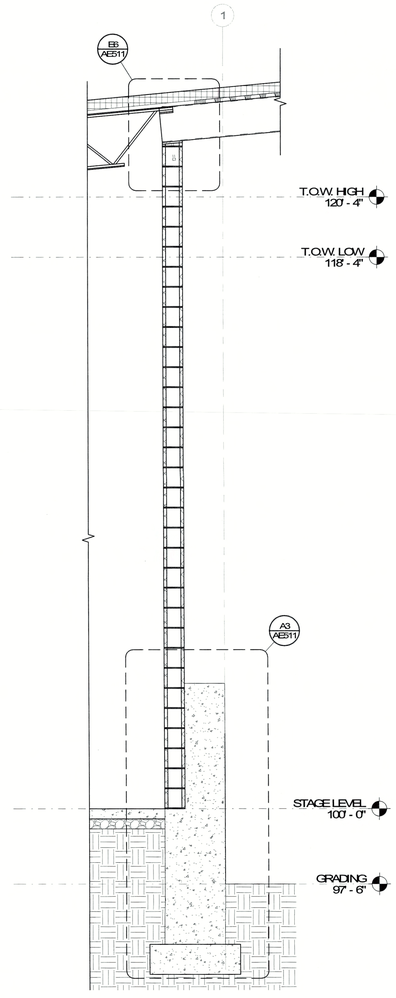
A2 WALL SECTION
1/2" = 1'-0"



A3 WALL SECTION
1/2" = 1'-0"



A4 WALL SECTION
1/2" = 1'-0"



A5 WALL SECTION
1/2" = 1'-0"

LICENSED ARCHITECT
5610739-0301
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STATE OF UTAH

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project:
North Ogden Amphitheater

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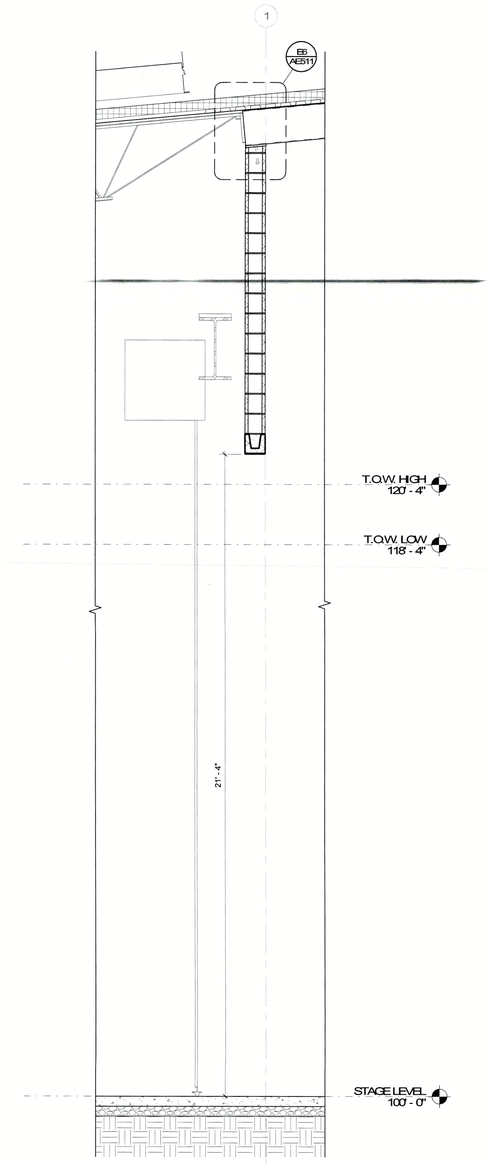
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revisions:

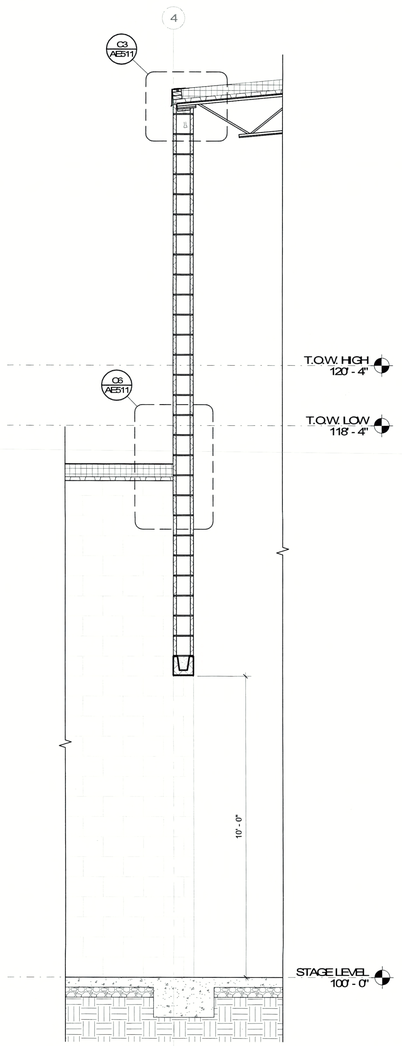
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Wall Sections

sheet:
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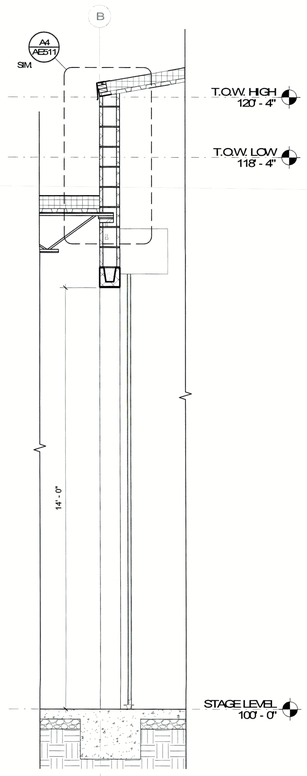
Phase 1 Bid Package 2



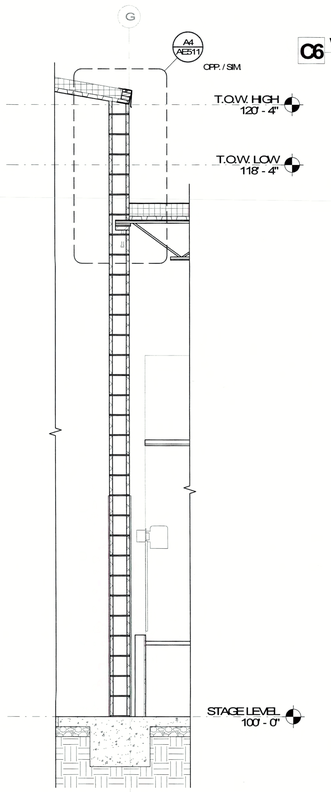
A1 WALL SECTION
1/2" = 1'-0"



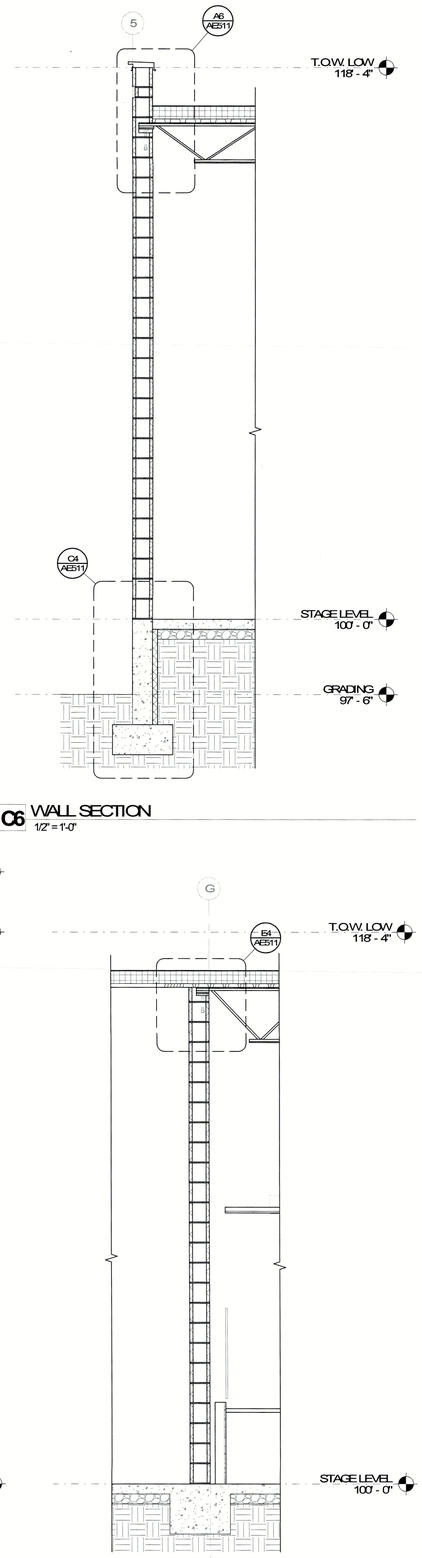
A2 WALL SECTION
1/2" = 1'-0"



A4 WALL SECTION
1/2" = 1'-0"



A5 WALL SECTION
1/2" = 1'-0"



A6 WALL SECTION
1/2" = 1'-0"

C6 WALL SECTION
1/2" = 1'-0"

LICENSED ARCHITECT
6610739-0301
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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

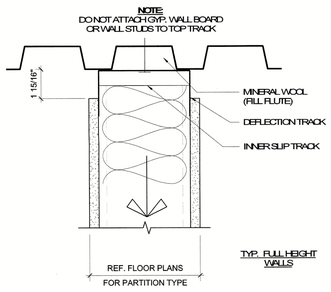
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date: 12-1-17

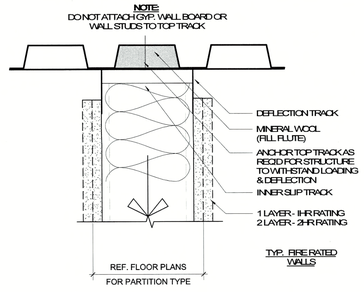
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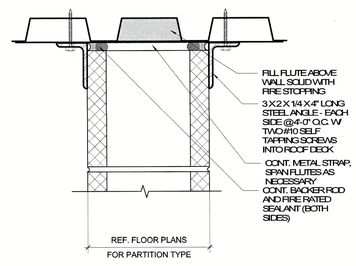
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Wall Sections

sheet:
AE312

Phase 1 Bid Package 2

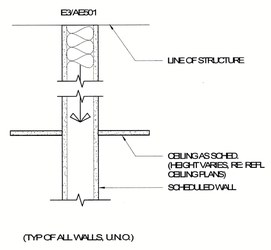


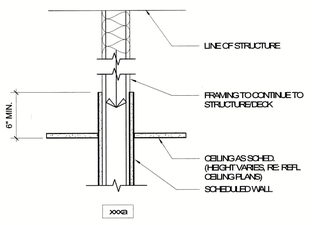


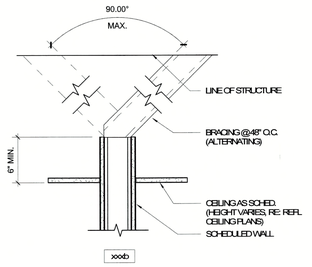


E3 Wall Types - deflection details

3" = 1'-0"

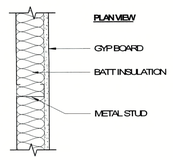






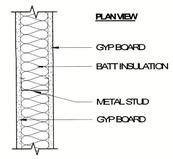
WALL TYPES - head condition

1 1/2" = 1'-0"



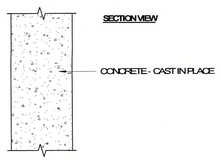
RS3 3 5/8" STEEL STUD
RS6 6" STEEL STUD

RS PARTITION FURRED WALL



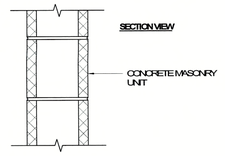
PS3 3 5/8" STEEL STUD
PS6 6" STEEL STUD
PS10 10" STEEL STUD

PS_ PARTITION METAL STUD WALL



C8 8" CONCRETE

C_ CONCRETE WALL



M8 8" x 8" x 16" CMU

M_ MASONRY WALL

WALL TYPES GENERAL NOTES

1. REFER TO FLOOR PLAN "AE" SERIES FOR LOCATION OF WALL TYPES.
2. REFER TO SCHEDULES & DETAILS FOR FINISHES.
3. "LINE OF STRUCTURE" AS SHOWN AT THE HEAD CONDITIONS OF EACH WALL TYPE IS DIAGRAMMATIC ONLY AND DOES NOT INDICATE THE EXACT CONSTRUCTION CONDITION. RATED WALLS ARE TO TERMINATE AT STRUCTURAL MEMBERS WITH A FIRE RESISTANT RATING. WHERE REQD APPROPRIATE FRAMING AND GYP BOARD IS TO BE INSTALLED AND OFFSET AROUND STRUCTURAL MEMBERS OR OTHER OBSTRUCTIONS SUCH AS PIPING OR DUCTWORK, TO MAINTAIN THE FIRE RESISTANCE RATING. NON-RATED WALLS THAT CONTINUE TO STRUCTURE ARE TO TERMINATE AT PROPER LOCATIONS TO MAINTAIN THE INTENT OF THE CONTINUOUS PLANE OF ONE LAYER OF GYP BOARD AS A NOISE, SMOKE OR OTHER TYPE OF BARRIER.
4. ALL GYP BOARD SHALL BE 5/8", UNLESS NOTED OTHERWISE.
5. ALL RATED WALLS SHALL BE CONSTRUCTED FIRST. SECONDARY WALLS SHALL ABUTT, BUT NOT PENETRATE RATED WALLS.
6. APPROPRIATE SUBMITTAL INFORMATION MUST BE PROVIDED TO SUBSTANTIATE THAT THE MATERIALS AND ASSEMBLY USED BY THE CONTRACTOR HAVE BEEN TESTED BY A RECOGNIZED TESTING AGENCY TO MEET THE FIRE RESISTANCE RATING SCHEDULED ON THESE WALL TYPES.
7. FIRESTOPPING TO BE PROVIDED AT PENETRATIONS THROUGH RATED WALLS AS SPECIFIED.
8. ALL GYPSUM WALL BOARD MUST BE MOISTURE RESISTANT AT THE FOLLOWING LOCATIONS:
   a. TOILET ROOMS
   b. WET WALLS
   c. SHOWERS
   d. JANITOR'S CLOSETS
9. SOUND ATTENUATION BLANKETS SHALL EXTEND THE FULL HEIGHT OF THE WALLS.
10. SPACING OF THE METAL STUDS HAS NOT BEEN INDICATED ON THE WALL TYPES OR DETAILS. STUD SPACING IS TO BE DETERMINED BY THE HEIGHT OF THE PARTITION AS SHOWN IN THE TABLE BELOW. EACH STUD GOING TO STRUCTURE AND EXCEEDING ALLOWABLE HEIGHTS SHALL BE BRACED 45 DEGREES DIAGONALLY 12" ABOVE CEILING WITH EQUAL SIZE 20 GA. METAL STUDS. THIS TABLE IS TO BE USED FOR THE INTERIOR WALL TYPES ONLY AND DOES NOT APPLY TO EXTERIOR STUDS. USE 20 GA STUDS AT ALL HEAD AND JAMB LOCATIONS.

SIDES OF STUDS	SPACING ON CENTER	STUD DEPTH 2 1/2" MAX. HT.	STUD DEPTH 3 5/8" MAX. HT.	STUD DEPTH 6" MAX. HT.
1 LAYER 5/8" GYP BD - 1 SIDE ONLY	16	11'-0"	14'-6"	14'-6"
	24	9'-9"	12'-9"	12'-9"
1 LAYER 5/8" GYP BD - EACH SIDE	16	12'-0"	16'-0"	16'-0"
	24	10'-9"	13'-6"	13'-6"
2 LAYER 5/8" GYP BD - EACH SIDE	16		16'-6"	20'-0"
	24		13'-6"	15'-0"

- HEIGHT IS DISTANCE FROM THE FLOOR TO THE STRUCTURE, NOT FLOOR TO CEILING
- BRACING AT MIDPOINT PREQUIRED FOR ALL WALLS OVER 12'-0" HIGH

11. REFER TO INTERIOR DETAILS FOR ADDITIONAL INFORMATION.
12. UL DESIGN NUMBERS REFER TO FIRE RESISTANCE IN MOST CURRENT EDITION OF THE UL DIRECTORY.
13. SUPPORT INSULATION WITH CHICKEN WIRE IN PARTITIONS WITHOUT GYP BOARD ON BOTH SIDES TO STRUCTURE.
14. MAINTAIN 1/2" SPACE BETWEEN FLOOR SLAB AND BOTTOM OF GYP BOARD ON ALL WALLS.
15. STOP STUD 1" BELOW METAL RUNNER (TOP TRACK) TO ALLOW FOR VERTICAL EXPANSION. FACING OUT ATTACH STUDS TO TOP TRACK ONLY.

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5610739-0301
KELLY BOYD MORGAN
STATE OF UTAH
12-1-2017

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2375 Fruitland Dr, North Ogden, UT 84414

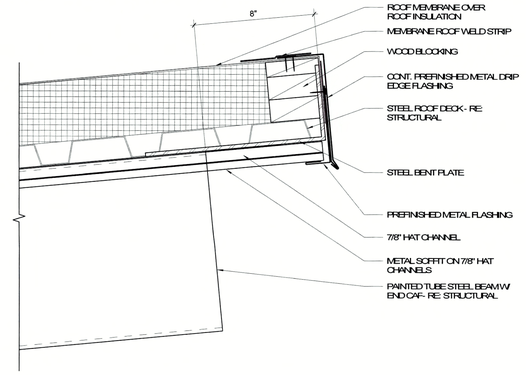
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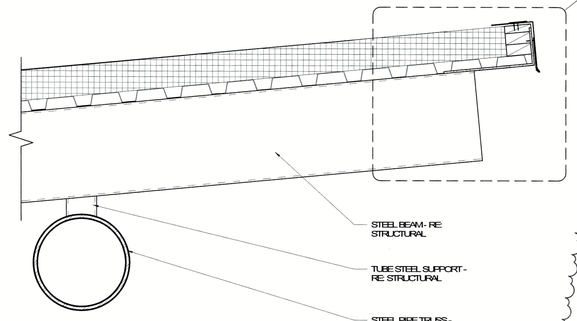
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Wall Types

sheet:
AE501

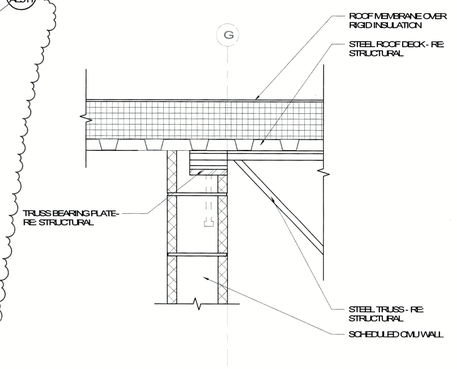
Phase 1 Bid Package 2



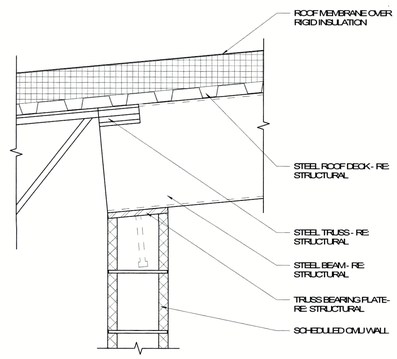
E1 ROOF EDGE DETAIL
3" = 1'-0"



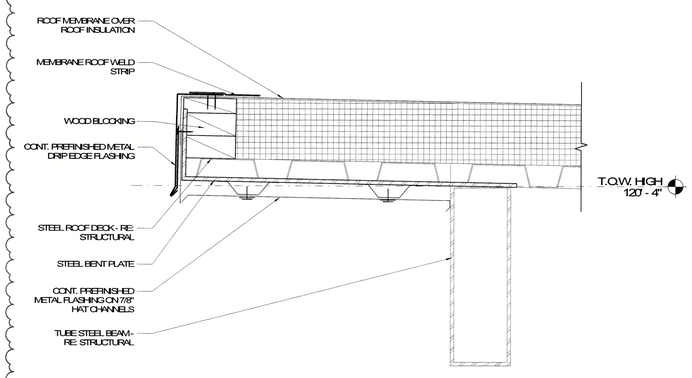
E3 ROOF EDGE DETAIL
1 1/2" = 1'-0"



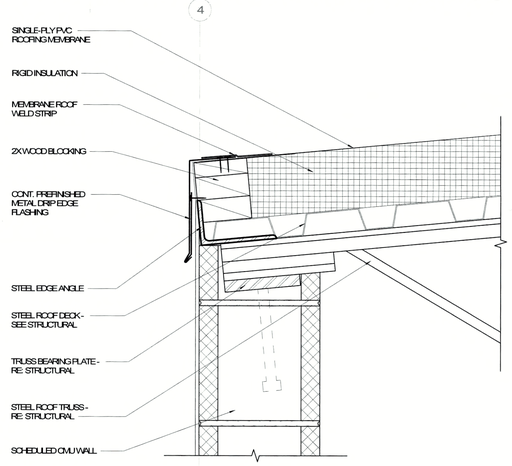
E4 SECTION DETAIL
1 1/2" = 1'-0"



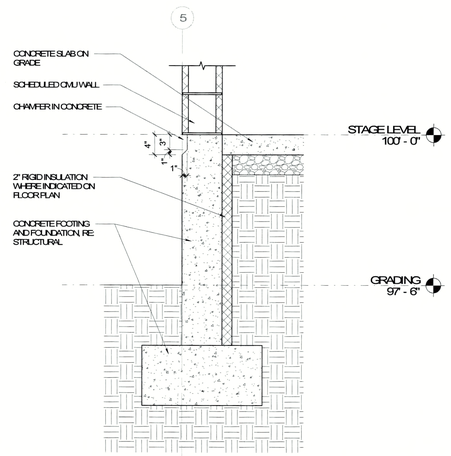
E6 SECTION DETAIL
1 1/2" = 1'-0"



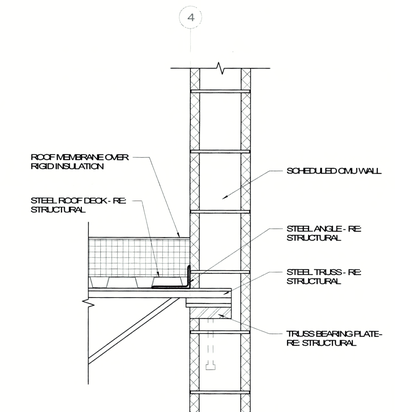
C1 ROOF EDGE DETAIL
3" = 1'-0"



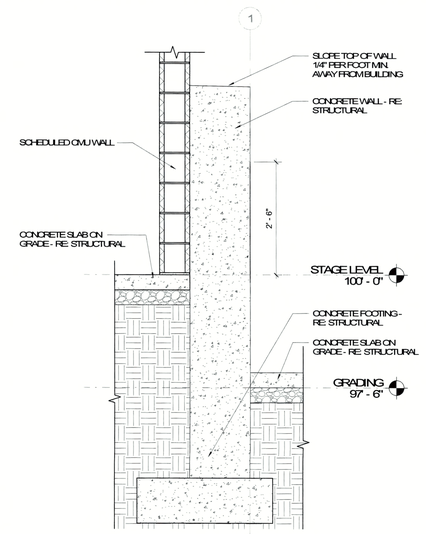
C3 ROOF EDGE DETAIL
3" = 1'-0"



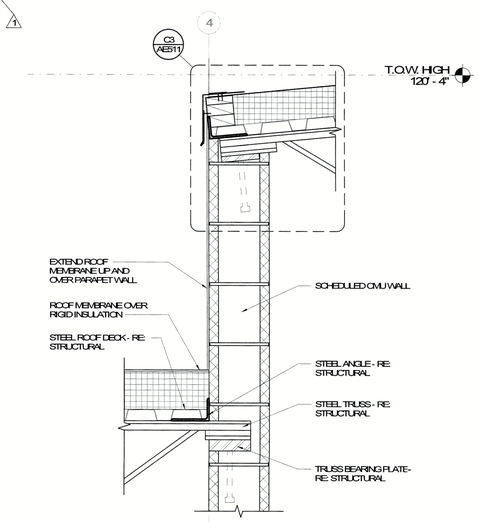
C4 FOOTING DETAIL
1" = 1'-0"



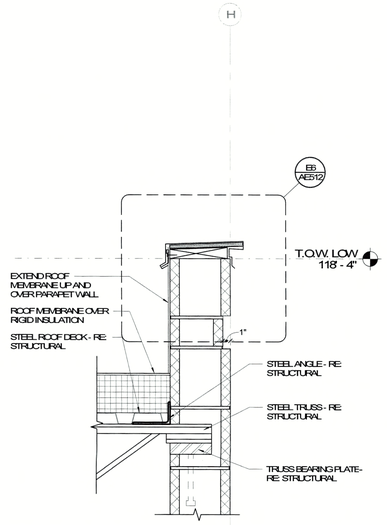
C6 SECTION DETAIL
1 1/2" = 1'-0"



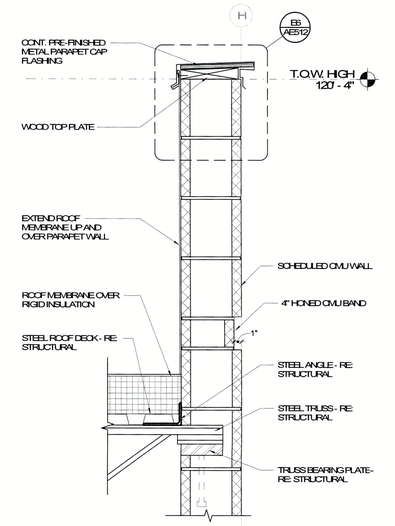
A2 FOOTING DETAIL
3/4" = 1'-0"



A3 SECTION DETAIL
1 1/2" = 1'-0"



A5 PARAPET DETAIL LOW
1 1/2" = 1'-0"



A6 PARAPET DETAIL HIGH
1 1/2" = 1'-0"

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9610739-0301
KELLY BOYD MORGAN
STATE OF UTAH
12-1-2017

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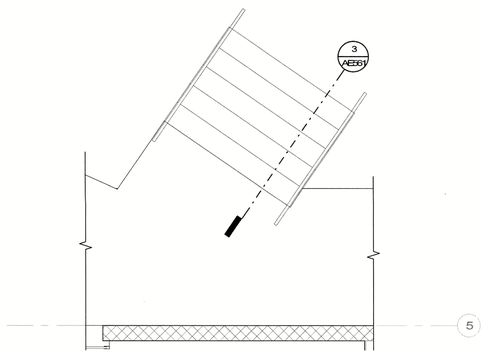
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date: 12-1-17

revisions:
1 PHASE 1 BP2 ADD 1 12/8/17

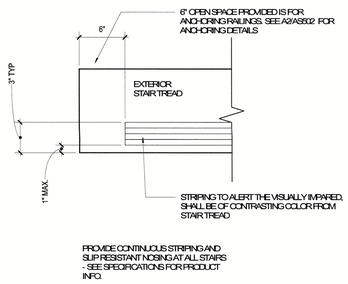
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Wall Section Details

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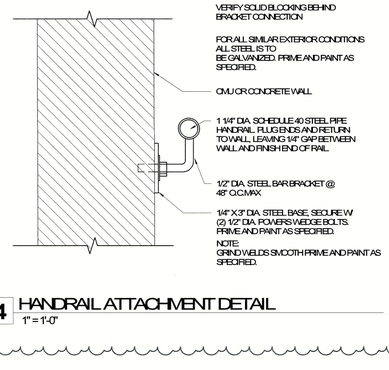
Phase 1 Bid Package 2



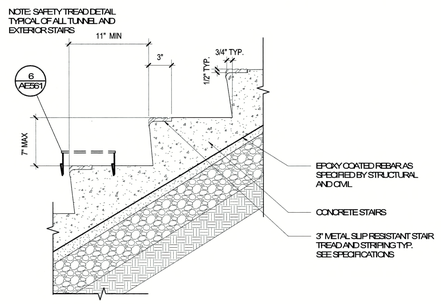
7 ENLARGED DOCK STAIR PLAN
3/8" = 1'-0"



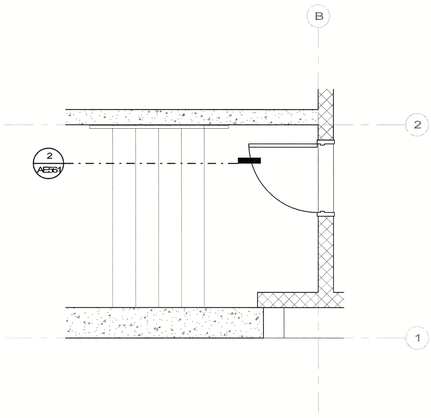
6 TYP STAIR TREAD DETAIL
1 1/2" = 1'-0"



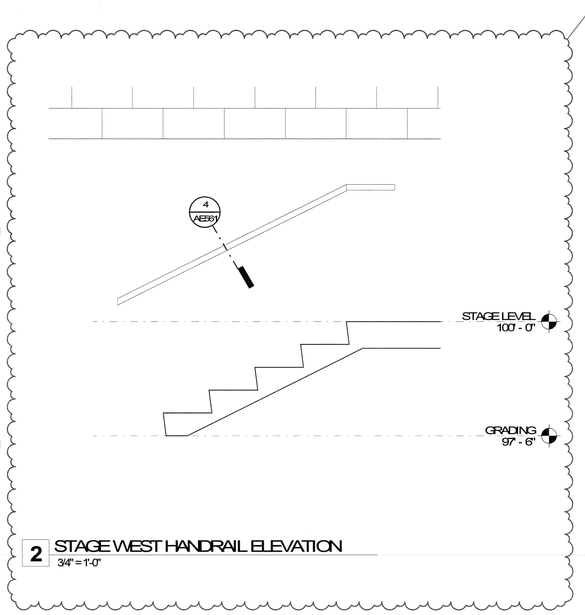
4 HANDRAIL ATTACHMENT DETAIL
1" = 1'-0"



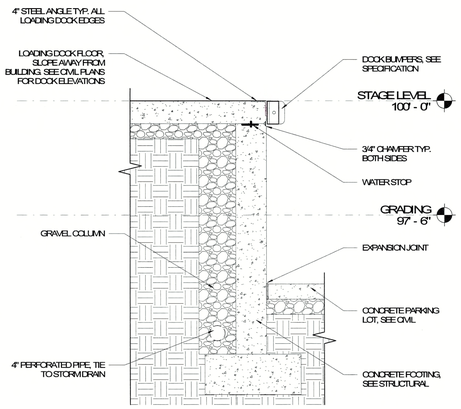
5 ENLARGED STAIR DETAIL
1 1/2" = 1'-0"



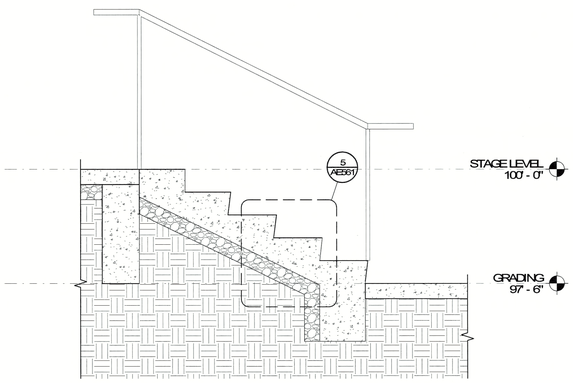
C1 ENLARGED STAGE WEST EGRESS STAIR
3/8" = 1'-0"



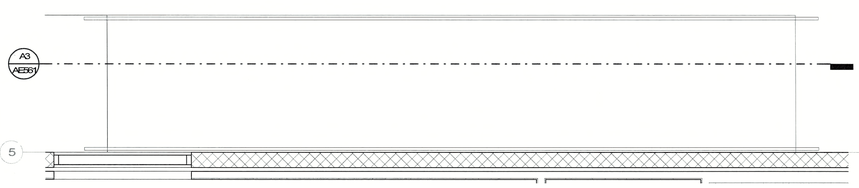
2 STAGE WEST HANDRAIL ELEVATION
3/4" = 1'-0"



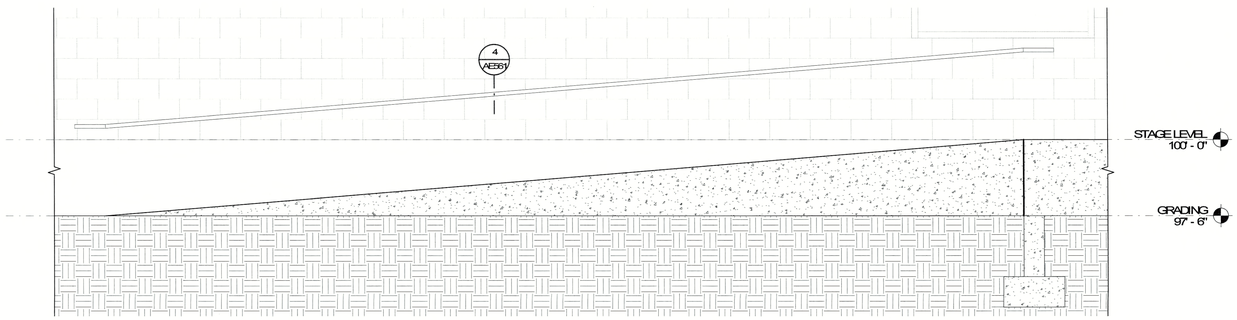
1 DOCK DETAIL
3/4" = 1'-0"



3 STAIR DETAIL
3/4" = 1'-0"



A1 ENLARGED DOCK RAMP PLAN
3/8" = 1'-0"



A3 DOCK RAMP SECTION
1/2" = 1'-0"

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5610739-0301
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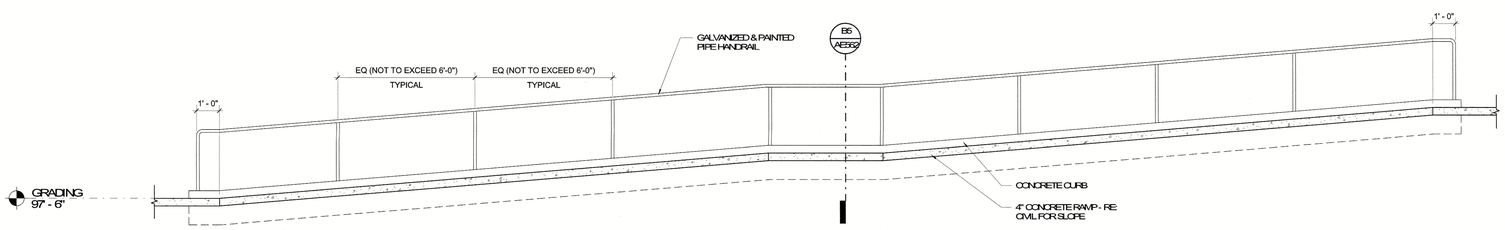
revisions:
1 PHASE 1 BP2 ADD 1 12/8/17

title:
Stair & Ramp Details

sheet:
AE561

Phase 1 Bid Package 2

B5 ADA Ramp @ Seating Detail
1/2" = 1'-0"



A1 ADA Ramp @ Seating
3/8" = 1'-0"

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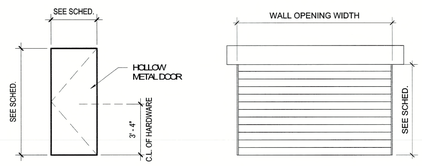
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Stair & Ramp Details

sheet:
AE562

Phase 1 Bid Package 2

DOOR SCHEDULE AMPHITHEATER

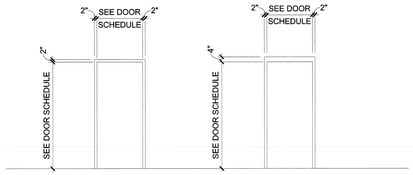
Number	Door Dimensions			Door			Frame			Details			Fire Rating	Hardware Set	Comments
	Width	Height	Thickness	Type	Material	Finish	Type	Material	Finish	Head	Jamb	Sill			
A110	3' - 8"	7' - 0"	1 3/4"	D1	HOLLOW METAL	PAINT	F2	HOLLOW METAL	PAINT	A4 AE601	A4 AE601	A4 AE601	-	01	
A113B	12' - 0"	10' - 0"	2"	D2	SECTIONAL STEEL	-	-	-	-	A5 AE601	A5 AE601	A5 AE601	-	00	Motorized
A116	3' - 0"	7' - 0"	1 3/4"	D1	HOLLOW METAL	PAINT	F2	HOLLOW METAL	PAINT	A4 AE601	A4 AE601	A4 AE601	-	01	
A117	40' - 0"	22' - 0"	2"	D2	SECTIONAL STEEL	-	-	-	-	A5 AE601	A5 AE601	A5 AE601	-	00	Motorized
A118	3' - 0"	7' - 0"	1 3/4"	D1	HOLLOW METAL	PAINT	F2	HOLLOW METAL	PAINT	A4 AE601	A4 AE601	A4 AE601	-	01	



D1
HOLLOW METAL

D2
OVERHEAD COILING DOOR

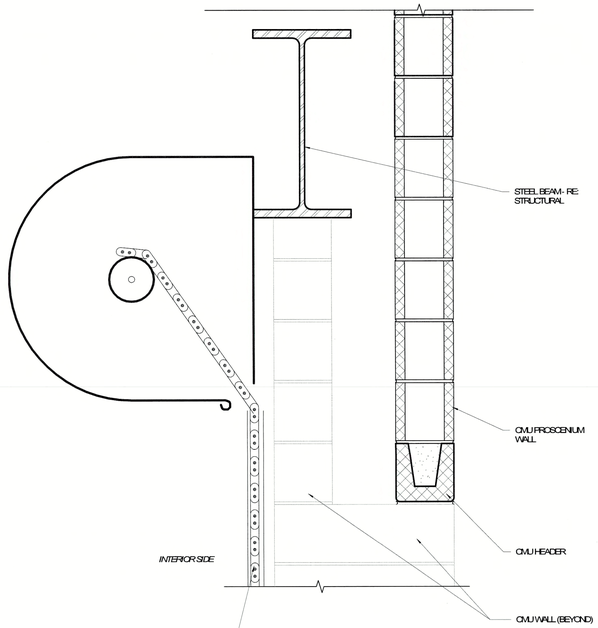
C4 Door Types
1/4" = 1'-0"

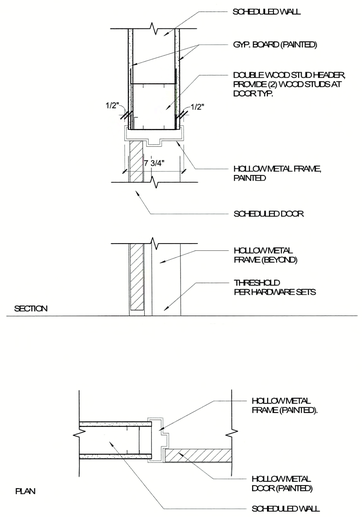


F1
HM

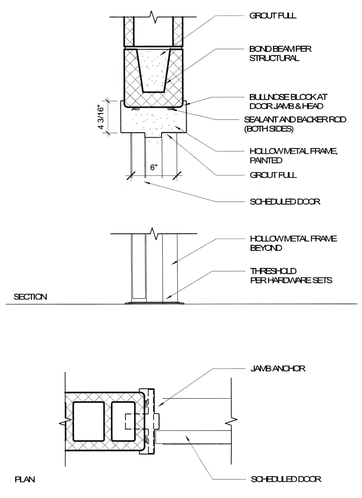
F2
HM

C5 Door Frame Types
1/4" = 1'-0"

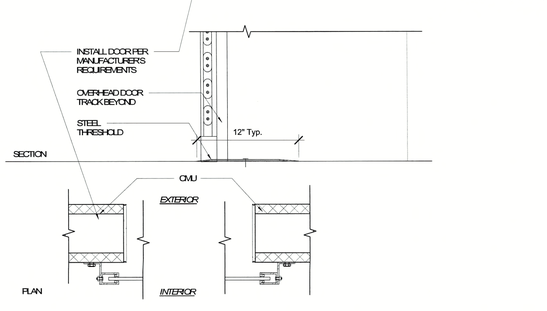




A3 Typical HM Interior Door
1 1/2" = 1'-0"



A4 Typical HM Door @ CMU
1 1/2" = 1'-0"



A5 Roll Up Door Head
1 1/2" = 1'-0"

LICENSED ARCHITECT
5610739-0301
KELLY BOYD MORGAN
STATE OF UTAH
12-1-2017

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

project#: 17.0555
date: 12-1-17

revisions:

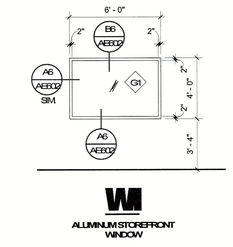
title:
Door Schedule, Types, Details

sheet:
AE601

Phase 1 Bid Package 2

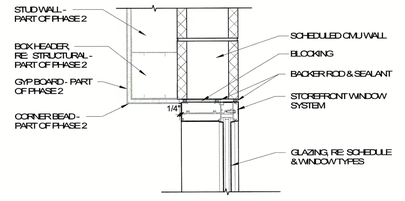
GLAZING/SPANDREL SCHEDULE

Type	Type
G1	1" MIRRORED ONE WAY GLAZING

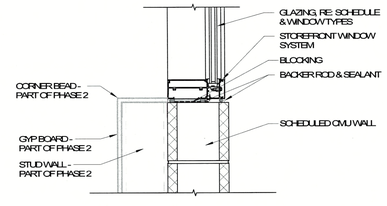


W ALUMINUM STOREFRONT WINDOW

C6 Window Types
1/4" = 1'-0"



B6 Window SF Head Detail @CMU
1 1/2" = 1'-0"



A6 Window SF Sill Detail @CMU
1 1/2" = 1'-0"

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5610739-0301
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12-1-2017

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

project#: 17.0555
date: 12-1-17

revisions:

title:
Window Schedule, Types, Details

sheet:
AE602

Phase 1 Bid Package 2

GENERAL STRUCTURAL NOTES

GENERAL

1. The structural notes are intended to complement the project specifications. Specific notes and details in the drawings shall govern over the structural notes and typical details.
2. Typical details and sections shall apply where specific details are not shown.
3. The contractor shall verify all site conditions and dimensions. If actual conditions differ from those shown in the contract drawings, the contractor shall immediately notify the architect/engineer before proceeding with the fabrication or construction of any affected elements.
4. Omissions or conflicts between the contract drawings and/or specifications shall be brought to the attention of the architect/engineer before proceeding with any work involved. In case of conflict, follow the most stringent requirement as directed by the architect/engineer at no additional cost to the owner.
5. The contractor shall submit a written request to the architect/engineer before proceeding with any changes, substitutions or modifications. Any work done by the contractor before receiving written approval will be at the contractor's risk.
6. The structural drawings are not all-inclusive and do not contain all dimensions, elevations, openings, mechanical shafts and penetrations needed to build the structure. The contractor shall coordinate these items with the Architectural, Mechanical and Electrical drawings.
7. The contractor shall coordinate with all trades any items that are to be integrated into the structural system such as openings, penetrations, mechanical and electrical equipment, etc. Sizes and locations of mechanical and other equipment that differs from those shown on the contract drawings shall be reported to the architect/engineer.
8. The contractor shall provide adequate shoring and bracing as required for his method of erection. Shoring and bracing shall remain in place until final connections for the permanent members are completed. The building shall not be considered stable until all connections are completed. Walls shall not be considered self-supporting and shall be braced until the roof system is completed.
9. The contractor shall not cut or core any holes in masonry or concrete walls without prior review by the architect/engineer.
10. Site observations by BHB Consulting Engineers, P.C.'s field representative shall not be construed as approval of construction procedures nor special inspection.
11. Detailing and shop drawing production for structural elements will require information (including dimensions) contained in the architectural, structural and/or other consultants' drawings. The structural drawings shall be used in conjunction with the architectural and other consultant's drawings. Some dimensions and elements such as elevations, depressions, slopes, mechanical housekeeping pads, etc. are not shown in the structural drawings. All dimensions shown on structural drawings shall be verified by contractor with architectural, mechanical and electrical drawings.
12. Review of shop drawing submittals by BHB Consulting Engineers, P.C. is for general compliance only and is not intended for approval. The shop drawing review shall not relieve the contractor from the responsibility of completing the project according to the contract documents.
13. Shop drawings made from reproductions of the contract drawings will be rejected unless the contractor signs a release agreement prior to the shop drawings being reviewed.
14. Only an authorized representative of BHB Consulting Engineers, P.C. may make changes to these contract drawings. BHB Consulting Engineers, P.C. shall not be held responsible or liable for any claims arising directly or indirectly from changes made without written authorization by an authorized representative of BHB Consulting Engineers, P.C.
15. Bidding, pricing or construction done prior to receiving final building permits from the authorities having jurisdiction is at the contractors own risk. Changes to the drawings may be required as part of the plan check process. BHB Consulting Engineers P.C. will not be held liable for, nor compensate for, changes to these drawings before final jurisdiction approval is obtained.

BASIS OF DESIGN

1. Governing Code — International Building Code 2015
   a. Risk Category — II
2. Snow Loads
   a. Ground Snow Load — P_g = 43 psf
   b. Snow Importance Factor — I_s = 1.0
   c. Snow Exposure Coefficient — C_e = 1.0
   d. Thermal Exposure Coefficient — C_t = 1.0/1.2
   e. Roof Snow Load — $P_f = 0.7 * C_e * C_t * I_s * P_g$ = 30/36 psf plus Snow Drift
3. Seismic Loads
   a. Seismic Importance Factor, I_e — 1.0
   b. Seismic Design Category — D
   c. Mapped Spectral Acceleration — S_s = 1.57g; S_1 = 0.56g
   d. Soil Site Class — D
   e. Soil Site Coefficients — F_a = 1.0; F_v = 1.5
   f. 5% Damped Design Spectral Response Acceleration — $S_{DS} = 2/3 * F_a * S_S$ = 1.04g; $S_{D1} = 2/3 * F_v * S_1$ = 0.56g
   g. Seismic-Force-Resisting System — Special Reinforced Masonry Shear Walls
   h. Response Modification Coefficient — R = 5.0
   i. System Over-strength Factor — Ω = 2.5
   j. Deflection Amplification Factor — C_d = 3.5
   k. Fundamental Building Period — T = 0.14 seconds
   l. Seismic Response Coefficient — $Cs = SDS * I_e / R$; $Cs = SD1 * I_e / (R*T)$
   m. W — Dead Loads of Structure
   n. Base Shear — $V = Cs * W$ = 0.24 W (Strength Design)
   o. Analysis Procedure — Equivalent Lateral Force (Static)
4. Wind Loads
   a. Wind Velocity (3 Second Gust) — 115 mph (Strength); 90 mph (Allowable (I_w = 1.0))
   b. Exposure Type — C
   c. Internal Pressure Coefficient, GCpi — +/-0.18
   d. Topographic Factor, Kzt — 1.0
   e. Components and Cladding Wind Force Table (psf; Strength Design)

Component Elevation above grade	Effective Wind Area for Component (sq ft.)				
	10 sq ft.	20 sq ft.	50 sq ft.	100 sq ft.	500 sq ft.
15	-38.6	-36.0	-32.6	-30.0	-23.9
20	-41.0	-38.2	-34.6	-31.8	-25.4
25	-43.0	-40.1	-36.3	-33.4	-26.7

FOUNDATION

1. Soils Report
   a. Author — Christensen Geotechnical,
   b. Dated — August 7, 2017
   c. Project No 121-001
2. Soil Bearing Pressure: 3000 psf under gravity loads with a 1/3 increase under short term lateral loads, on Soil Improved with Rammed Aggregate Piers
3. Frost Protection: 30 inches minimum to bottom of footing.
   Contractor shall field verify that the footing elevations and final grades indicated on the plans will provide the minimum frost protection. The contractor shall notify the architect/engineer if there are any locations where the minimum frost protection might not be achieved prior to placing concrete.sd
4. Lateral Soil Pressure Fluid Equivalent Density.
   a. Active — 45 pcf (retaining walls)
   b. At Rest — 65 pcf (rigid foundation walls)
   c. Passive — 295 pcf
5. Coefficient of Friction — 0.31

EARTHWORK

1. All footings shall bear on soil improved with Rammed Aggregate Piers.
2. Slabs on grade may be placed on properly prepared existing near-surface soil, suitable undisturbed natural material or on structural fill extending down to suitable natural material. Follow soils report recommendations for preparation. Use pavement requirements in the parking garage.
3. All foundation and retaining walls shall be back filled with gravel or native sand and gravel. No native clay material may be used to backfill.
4. Consult the project specifications and soils report for further earthwork requirements.

RAMMED AGGREGATE PIERS

1. Soil shall be improved under all foundations with rammed aggregate piers.
2. Soil bearing pressure after improvement shall be 3000 psf under gravity loads with a 1/3 increase under short term lateral loads
3. Consult the project specifications and soils report for further earthwork requirements.

CONCRETE

1. Materials, unless noted otherwise:
   a. Normal weight aggregates — ASTM C 33
      i. Combined aggregate gradation for slabs on grade and other designated concrete shall be 8% - 18% for large top size aggregates (1½ in.) or 8% - 22% for smaller top size aggregates (1 in. or ¾ in.) retained on each sieve below the top size and above the No. 100. The range for the No. 30 and No.50 sieves shall be 8% - 15% retained in each. To avoid gap gradation the following shall occur:
         1. The percent retained on two adjacent sieves shall not fall below 5%.
         2. The percent retained on three adjacent sieves shall not fall below 8%.
         3. When the percent retained on two adjacent sieves is less than 8%, the total retained on either of these sieves and the adjacent outside sieve shall be at least 13%. See ACI 302 Section 5.4.3.3 for more information.
      ii. A gradation chart or table shall be submitted with mix designs for slabs on grade.
      iii. Maximum Aggregate Size shall be not be larger than:
         1. 1/5 the narrowest dimension of the forms
         2. 1/3 the depth of the slab
         3. ¾ the minimum clear spacing between bars
   b. Reinforcing Steel — ASTM 615 Grade 60 (Fy = 60 ksi). Use Grade 40 (Fy = 40 ksi) for field bent dowels with spacings indicated reduced by 1/3.
   c. Deformed Bar Anchors (DBA) — ASTM A496
   d. Headed Stud Anchors (HSA) — ASTM A108
   e. Anchor Rods Typical, uno — ASTM F1554, Grade 36, with ASTM A563 heavy hex nuts and hardened washers Grade A
   f. Admixtures:
      i. Air-entraining admixtures shall comply with ASTM C 260 (when used).
      ii. Calcium chloride shall not be added to the concrete mix.
      iii. Water-reducing admixture shall comply with ASTM C 494/C 494M, Type A (when used)
      iv. Retarding admixture shall comply with ASTM C 494/C 494M, Type B (when used).
      v. Water-reducing and retarding admixture shall comply with ASTM C 494/C 494M, Type D (when used).
      vi. High-range, water-reducing admixture shall comply with ASTM C 494/C 494M, Type F (when used).
      vii. High-range, water-reducing and retarding admixture shall comply with ASTM C 494/C 494M Type G (when used).
      viii. Admixture manufacturer shall have ISO 9001 Quality Certification. To ensure compatibility all admixtures shall be from the same manufacturer.
   g. Type I/II cement complying with ASTM C-150 shall be used for all concrete. Cement source shall remain the same for the entire job.
   h. Blended cement Type GU complying with ASTM C1157
   i. Fly Ash - ASTM C618, Class F – 25% maximum cementitious content
   j. The water/cementitious materials ratios shall meet the requirements of Table 4.3.1 of ACI 318.
   k. Provide air entraining as recommended by Table 4.4.1 of ACI 318. Concrete that extends above grade and is exposed to freezing and thawing while moist shall be air-entrained.
   l. No aluminum conduit or product containing aluminum or any other material injurious to concrete shall be embedded in concrete.
2. Compressive strengths of concrete at 28 days shall be as follows
   a. Interior Footings& Interior Foundation Walls — Strength 3,000 psi; Classification F0, S0, W0, C0
   b. Exterior Footings & Exterior Foundation Walls — Strength 3,000 psi; Classification F0, S0, W0, C0
   c. Interior Slabs on Grade — Strength 3,000 psi; Classification F0, S0, W0, C0
   d. All Site Concrete with Reinforcement — Strength 5,000 psi; Classification F3, S0, W1, C2
   e. All Site Concrete without Reinforcement — Strength 4,500 psi; Classification F3, S0, W1, C2
   f. Architectural Concrete (concrete walls) — Strength 4,500 psi; Classification F3, S0, W1, C1
3. Only one grade or type of concrete shall be poured on the site at any given time.
4. The contractor shall be responsible for the design, detailing, care, placement and removal of all formwork and shores.
   a. Supporting forms and shoring shall not be removed until structural members have acquired sufficient strength to safely support their own weight and any construction load to which they may be subjected. In no case, however, shall forms and shoring be removed in less than 24 hours after concrete placement.
5. Reinforcement shall have the following concrete cover:

	Clear Cover
a. Cast-in-place Concrete	
i. Cast against and permanently exposed to earth	3"
ii. Formed concrete exposed to earth or weather:	
#6 thru #18 bars	2"
#5 and smaller bars	1-1/2"
iii. Concrete not exposed to weather or in contact with ground:	
Slabs, Walls and their piers, Joists; #11 bars and smaller	3/4"
Beams, Columns: Primary Reinf., Ties, Stirrups, Spirals	1-1/2"

6. Construction Joints, Control (Contraction) Joints:
   a. Construction joints in all horizontal and vertical construction joints including between top of footing and foundation walls shall be intentionally roughened to a full amplitude of approximately 1/4 inch. The laitance on the concrete (thin, flaky layer of hardened but weak hydrated cement) shall be mechanically removed from the surface after the concrete has achieved final set.
   b. Control joints shall be installed in slabs on grade so the length to width ratio of the slab is no more than 1.25:1. Control joints shall be completed as soon as final set is achieved and it is okay to operate the cutter on the slab. Final set is typically achieved within the first 4 to 6 hours of the slab pour. For early entry saw cutting, joints should be cut within the first 1 to 4 hours, depending on weather conditions and concrete hydration rate. Where saw cut joints cannot be cut along the entire projected length of the joint, a 90 degree hand grinder or other tool shall be used to complete the joint. Control joints may be installed by:
      i. Saw cut a depth of 1/4 the thickness of the slab (1 1/4" ± for early entry saws)
      ii. Tooled joints a depth of 1/4 the thickness of the slab
      iii. Saw cut depth shall be increased to 1/3 of the slab thickness (1 3/4" ± for early entry saws) where macro fibers are used.
   c. For interior concrete slabs-on-grade that are to receive **no** floor covering, install construction or control joints in slabs on grade at a spacing not to exceed 30 times the slab thickness in any direction, unless noted otherwise. For interior concrete slabs-on-grade that are to receive floor coverings the contractor has the option to eliminate control joints. Construction joints shall not exceed a distance of 125'-0" o.c. in any direction.
7. Construction
   d. Use chairs or other support devices recommended by the CRSI to support and tie reinforcement bars prior to placing concrete. Reinforcing steel for slabs on grade shall be adequately supported. Support reinforcing steel of slabs on grade with precast concrete units. Lifting the reinforcing off the grade during placement of concrete is not permitted.
   e. Concrete to be mechanically consolidated during placement per ACI standards.
   f. Contractor shall coordinate placement of all openings, curbs, dowels, sleeves, conduits, bolts, inserts and other embedded items prior to concrete placement.
   g. All embeds, anchors and dowels shall be securely tied to formwork or to adjacent reinforcing prior to the placement of concrete.
   h. No pipes, ducts, sleeves, etc shall be placed in structural concrete unless specifically detailed or approved by the structural engineer. Penetrations through walls when approved shall be built into the wall prior to concrete placement. Penetrations will not be allowed in footings or grade beams unless detailed. Piping shall be routed around footings and grade beams and unless detailed. Footings shall be stepped to avoid piping.
   i. Reinforcing Bars shall not be welded. Do not substitute reinforcing bars for DBAs or HSAs.
8. Detailing:
   a. Lap splice lengths shall be detailed to comply with the "Concrete Reinforcing Bar Lap Splice Schedule" on sheet S601. Splices may be made with mechanical splices capable of 125% tension capacity of the bar being spliced. Mechanical splices shall be the positive connecting type coupler and shall meet all International Building Code requirements and shall have a current ICC-ES report or IAPMO Certification. Use "Lenton" Standard Couplers (ICC ER-3967), "Bar-Lock" (ICC ESR-2495) or equal with internal protector. If mechanical splices are used, splices or couplers on adjacent bars shall be staggered a minimum of 24" apart along the longitudinal axis of the reinforcing bars.
   b. At joints provide reinforcing dowels to match the member reinforcing, unless noted otherwise.
   c. At all discontinuous control or construction slab on grade joints, provide 2 - #4 x 48 inches.
   d. Corner Bars: Provide corner bars at intersecting wall corners using the same bar size and spacing as the horizontal wall reinforcing. Corner bars shall lap the horizontal reinforcing with the required lap splice length.
   e. All vertical reinforcing shall be doweled to footings, or to the structure below with the same size and spacing as the vertical reinforcing for the element above. Dowels extending into footings shall terminate with a 90 degree standard hook and shall extend to within 4" of the bottom of the footing. Footing dowels (#8 bars and smaller) with hooks need not extend more than 20" into footings.
   Horizontal wall reinforcing shall be continuous through construction and control joints.

POST-INSTALLED ANCHORS

1. Adhesive Anchors
   a. For concrete, the adhesive shall be HIT RE 500-SD (ICC-ES ESR-2322) by Hilti Inc., HIT-HY 200 with Safe Set™ Technology (ICC-ES ESR-3187) by Hilti Inc., Pure 110 + (ICC-ES ESR-3298) by Powers Fasteners Inc, SET-XP (ICC-ES ESR-2508) by Simpson Strong-Tie or AT-XP (IAPMO-UES ER-263) by Simpson Strong-Tie Inc., Sika AnchorFix-3001 (ICC-ES ESR-3608) by Sika Corporation.
   b. For grouted masonry, the adhesive shall be HIT-HY 70 (ICC-ES ESR-2682) by Hilti Inc., SET-XP (IAPMO UES ER-265) by Simpson Strong-Tie Inc. or AT-XP (IAPMO UES ER-281) by Simpson Strong-Tie Inc., AC100+ (ICC-ES ESR-3200) by Powers Fasteners Inc. or CIA GEL (ICC-ES ESR-1702) by USP.
   c. For ungrouted masonry, the adhesive shall be HIT-HY 70 (ICC-ES ESR-2682) by Hilti Inc., or SET (ICC-ES ESR-1772) by Simpson Strong-Tie Inc. or AC100+ (ICC-ES ESR-3200) by Powers Fasteners Inc. Plastic mesh or stainless steel screen tubes shall be used.
   d. Follow all of the manufacturer's recommendations and certification testing reports for adhesive installation.
   e. Alternative epoxies may be used if an ICC-ES ESR or IAPMO-UES ER approval for use in cracked concrete is submitted to the structural engineer prior to use.
2. Mechanical Anchors
   a. For concrete, the mechanical anchor shall be Kwik Bolt TZ (ICC-ES ESR-1917) by Hilti Inc., Strong-Bolt 2 (ICC-ES ESR-3037) by Simpson Strong-Tie Inc. or Power-Stud+ SD2 (ICC-ES ESR-2502) by Powers Fasteners Inc.
   b. For grouted masonry, the mechanical anchor shall be Kwik Bolt 3 (ICC-ES ESR-1385) by Hilti Inc., Wedge-All (ICC-ES ESR-1396) by Simpson Strong-Tie or Strong-Bolt 2 (IAPMO-UES ER-240) by Simpson Strong-Tie or Power-Stud+ SD1 (ICC-ES ESR-2966) by Powers Fasteners Inc.
   c. Follow all of the manufacturer's recommendations and certification testing reports for mechanical anchor installation.
   d. Alternative mechanical anchors may be used if an ICC-ES ESR or IAPMO-UES ER approval for use in cracked concrete is submitted to the structural engineer prior to use.
3. Screw Anchors
   a. For concrete and grouted masonry, the screw anchors shall be Titen HD (ICC-ES ESR-2713 for concrete only and ICC-ES ESR-1056 for grouted masonry) by Simpson Strong-Tie, or Screw Bolt + (ICC-ER ESR-3889 for concrete only) by DeWalt, Wedge-Bolt + (ICC-ES ESR-1678 for grouted masonry) by Powers Fasteners Inc, or Kwik HUS-EZ (ICC-ES ESR-3027 for concrete only and ICC-ES ESR-3056 for grouted masonry) by Hilti Inc.
   b. Follow all of the manufacturer's recommendations and certification testing reports for screw anchor installation.
   c. Alternative screw anchors may be used if an ICC-ES ESR or IAPMO-UES ER approval for use in cracked concrete is submitted to the structural engineer prior to use.
4. Powder Actuated Fasteners
   a. For fasteners driven into steel, the fastener shall be X-U P8 TH Universal Knurled Shank Fastener (ICC-ES ESR-2269) by Hilti Inc., PDPA (ICC-ES ESR-2138) by Simpson Strong-Tie Inc. or 8mm Head Spiral CSI Drive Pin (ICC-ES ESR-2024) by Powers Fasteners Inc.
   b. For fasteners driven into concrete or masonry, the fastener shall be X-U Universal Knurled Shank Fastener (ICC-ES ESR-2269) by Hilti Inc., PDP or PDPA (ICC-ESR-2138) by Simpson Strong-Tie Inc. or 8mm Head Spiral CSI Drive Pin (ICC-ES ESR-2024 for concrete only) by Powers Fasteners Inc.
   c. Follow all of the manufacturer's recommendations and certification testing reports for powder actuated fastener installation.
   d. Alternative powder actuated fasteners may be used if an ICC-ES ESR or IAPMO-UES ER approval for use in cracked concrete is submitted to the structural engineer prior to use.

MASONRY

1. Materials, unless noted otherwise:
   a. Concrete Masonry Units (CMU) ASTM C90: Lightweight Grade N (minimum net area unit strength of 2,000 psi). f'_m = 2,000 psi.
   b. Mortar Cement: Use Type "S"
   c. Masonry Grout ASTM C476: grout shall attain a minimum compressive strength of 2,500 psi at 28 days.
   d. Reinforcing Steel — ASTM 615 Grade 60 (Fy = 60 ksi)
   e. Deformed Bar Anchors (DBA) — ASTM A496
   f. Headed Stud Anchors (HSA) — ASTM A108
   g. Anchor Rods — ASTM F1554, Grade 36,with ASTM A563 heavy hex nuts and ASTM F436 hardened washers
2. Reinforcement shall have the following cover:
   a. Typical reinforcement shall have a minimum coverage of one bar diameter over all the bars, but not less than 3/4". When masonry is exposed to soil, minimum coverage shall be 1.5".
3. Construction Requirements:
   a. Masonry coursing shall be coordinated with the architectural drawings.
   b. All units shall be laid with full mortar beds on the face shells. All head joints shall be filled solidly with mortar for a distance in from the face of the units not less than the thickness of the longitudinal face shells. Cells which are to be grouted shall have full head joints.
   c. Masonry walls, beams and columns shall be constructed with running bond, unless noted otherwise.
   d. All cells containing reinforcement, embeds, anchor bolts, etc. shall be filled solid with grout. Grout shall be placed by mechanical vibration during placing and re-vibrated after excess moisture has been absorbed but before workability is lost. Rodding of grout is not allowed.
   e. Where walls are not grouted solid, each grout pour shall terminate flush with the top of the uppermost unit except at cells with vertical reinforcing where the grout shall be 1-1/2 inches below top of unit to provide construction key.
   f. Grout pours shall be limited to 4'-0" unless written approval is obtained from the engineer of record.
   g. All walls below grade shall be grouted solid.
   h. Vertical cells to be filled with grout shall have vertical alignment sufficient to maintain a clear, unobstructed vertical cell measuring not less than 2 inches by 3 inches. All steel reinforcement shall be secured against displacement prior to grouting by wire positioners or other suitable devices at intervals not exceeding 200 bar diameters or 10 feet maximum, or at bar splice locations. Vertical reinforcing shall be located at the center of the wall unless noted otherwise
   i. Reinforcing Bars shall not be welded. Do not substitute reinforcing bars for DBAs or HSAs.
   j. Control Joints: Spacing shall not exceed 30'-0". See architectural drawings for locations.
   k. Grout all beam and joist pockets solid after installation of beams and joists.
   l. Embed channels and plates shall be placed so as to create a flush surface with the face of the wall.
   m. Anchor bolts and headed stud anchors shall be set in a grouted cell. Anchor bolts and headed stud anchors shall have 1" grout surrounding the shank at its penetration. Grout shall be flush with the face or top of the masonry.
4. Detailing Requirement
   a. Lap all masonry reinforcing per "Masonry Reinforcing Lap Schedule" on sheet S602.
   b. All vertical reinforcing shall be doweled to the foundation wall, footing (structure below) and to the structure below with the same size dowel, spacing (and in the same core) as the vertical wall reinforcing above.
   c. Corner Bars: Provide corner bars at intersecting wall corners using the same bar size and spacing as the horizontal wall reinforcing. Corner bars shall lap the horizontal reinforcing with the required lap splice length.
   d. Wall Openings: For unscheduled openings wider than 24 inches, provide reinforcing on all sides per detail 8/S501. Also, for all scheduled openings, provide horizontal bar at bottom of opening per detail 8/S501. Vertical bars shall extend from floor level below to the floor, or roof level above. Horizontal bars for all openings shall extend a minimum of 48 bar diameters beyond the corners of the opening. Where a 48 bar diameter extension is not possible, extend bars as far beyond the opening as possible and terminate the bar(s) with a 90 degree standard ACI hook.
   e. Horizontal wall reinforcing shall be continuous through joining concrete walls, masonry walls, columns, and pilasters. Provide a key between the wall and the column or pilaster. Horizontal wall reinforcing shall be placed inside the column vertical reinforcing.
   f. Horizontal wall reinforcing shall terminate with a hook at edge of openings and at each side of control joints except at floor and roof levels, lintels, beams and at top of parapets. See details 10 and 11/S501.
   g. All masonry column ties shall terminate with 135 degree hooks plus a 6 bar diameter extension (4" minimum).

STRUCTURAL STEEL

1. Material:
   a. Wide Flanges Section — ASTM A992 (50 ksi)
   b. All Thread Rods, Other Shapes & Plates — ASTM A36 (36 ksi)
   c. Square, Rectangular, or Round HSS — ASTM A500 (46 ksi) Grade B or C
   d. Deformed Bar Anchors (DBA) — ASTM A496
   e. Headed Stud Anchors (HSA) — ASTM A108
   f. Non-Metallic Shrinkage Resistant Grout — ASTM C 1107
   g. Anchor Rods Gravity Columns — ASTM F1554, Grade 36, with ASTM A563 heavy hex nuts and ASTM F436 hardened washers Grade A
   h. Bolted Connections: — ASTM F3125 Grade A325 with ASTM A563 nuts and ASTM F436 hardened washers.
2. Fabrication and construction shall comply with the latest edition of the following Codes and Standards:
   a. American Institute of Steel Construction (AISC), "Specification for the Design, Fabrication and Erection of Structural Steel for Buildings," with "Commentary".
   b. AISC "Code of Standard Practice" excluding the following: Section 3.2, Section 4.4, Section 4.4.1,
   c. AISC "Specification for Structural Joints Using High-Strength Bolts"
   d. American Welding Society (AWS), Structural Welding Code (specific items do not apply when they conflict with the AISC requirements).
   e. AISC "Seismic Provision for Structural Steel Buildings"- ANSI/AISC 341
   f. All exterior steel elements, including anchor rods and bolts shall be hot-dip galvanized in accordance with ASTM A123 and A153 where applicable.
3. Welding
   a. Field weld flags that have been put in these documents are for suggestion only. The contractor has the option to substitute shop welding for field welding or vice versa. The steel fabrication and steel erection drawings must clearly distinguish between shop welds and field welds prior to any work being performed.
   b. Steel fabricators shall indicate the shop welds that are excluded from their bids. Steel erectors shall indicate the field welds that are excluded from their bids. It is the responsibility of the contractor to coordinate shop welding and field welding with the appropriate subcontractors.
   c. All welding and cutting shall be performed by AWS certified welders.
   d. Use E-70 XX or as noted otherwise. E60 XX may be used for welding steel roof decks.
   e. All intersecting steel shapes which are not bolted shall be connected by a fillet weld all around, unless noted otherwise. Where fillet weld sizes are not shown they shall be 1/16" less than the thinnest of the connected parts for thicknesses 1/4" and larger. Fillet welds on plates less than 1/4" shall be of the same size as the thinnest of the connected part.
   f. Reinforcing Bars: Do not weld rebar. Do not substitute reinforcing bars for deformed bar anchors (DBAs), machine bolts, or headed stud anchors (HSAs).
   g. Do not weld anchor bolts, including "tack" welds.
   h. Headed Stud Anchors (HSAs) welding and deformed bar anchor welding shall conform to the manufacturer's specifications.
4. Bolted Connections:
   a. Use ASTM A325N bolts for steel to steel connections, as noted herein or as noted on the drawings. A325N bolts shall be used in connections for simple span framing and beam (or girder) to bearing plate connections. Tighten bolts to a snug tight condition.
   b. Use hardened washers beneath the turned element of all bolts or nuts. Use hardened beveled washers, to compensate for the lack of parallelism, where the outer face of the bolted parts has a slope greater than one in twenty with respect to the plane normal to the bolt axis. At oversized holes hardened washers or plates shall conform with ASTM F-436 and shall completely cover the slot after installation.
   c. Where a steel to steel beam connection is not shown, provide a standard AISC framed connection for one half the total uniform load capacity of the beam for the span and steel specified.
   d. Bolts, nuts and washers shall not be reused.
5. Provide full-depth web-stiffener plates at each side of all beams at all bearing points. Stiffener plates shall be the thickness called out below unless noted otherwise and shall be welded both sides with fillet welds all around:

FLANGE WIDTH	STIFFENER THICKNESS	WELD SIZE
Less than 8 1/4"	1/4"	3/16"
8 1/4" to 12 1/4"	3/8"	1/4"
12 1/4" to 16 ½"	1/2"	5/16"
16 ½" to 20 ¾"	5/8"	3/8"

OPEN WEB STEEL JOISTS

1. All open web steel joist shall be fabricated and erected in accordance with the latest edition of Steel Joist Institute (SJI), "Standard Specifications and Code of Standard Practice".
2. At the completion of fabrication, the steel joist manufacturer shall submit to the building official a certificate of compliance in accordance with IBC Section 1704.2.5 stating if the work was performed in accordance with approved construction documents and with SJI standard specifications.
3. Joists with slopes greater than 1/2 inch per foot shall be designed to meet or exceed the load capacities, listed in the SJI load tables, of the joist sizes indicated on the framing plan, as if the joists or girders were installed level.
4. Provide special bearing ends to accommodate slopes from sloped joists, or sloped bearing conditions.
5. Modifications to any joist, including holes through the top and bottom chords, without the written consent and direction from the manufacturer are not allowed.
6. Joist loads called out in the drawings are allowable stress design (ASD) loads.
7. Open web joist deflection shall be limited to L/240 for total loads and L/360 for live load, unless noted otherwise.
8. Joist bridging shown on plans is for schematic purposes only; actual size, quantity and location of bridging shall be determined by the joist supplier per SJI. Coordinate bridging locations to avoid interference with mechanical, electrical and fire protection equipment and skylights.

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12-1-2017

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S001

Phase 1 Bid Package 2

GENERAL STRUCTURAL NOTES

METAL DECKING

1. Steel deck shall comply with the latest requirements of the Steel Deck Institute.
2. All deck shall be 3-span continuous minimum. In areas where 3-span conditions are not possible, the contractor shall provide heavier gage deck as required to provide the equivalent loading of the deck under a three span condition.
3. Steel roof deck shall not be used to support loads from plumbing, HVAC ducts, light fixtures, architectural elements or equipment of any kind, unless specifically noted. Light weight suspended acoustical ceilings with a total weight of 50 lbs per attachment may be hung from roof deck. The hangers shall be staggered to distribute the loads over multiple deck flutes.
4. All deck supporting members shall be dry before welding.
5. Clinch seams before welding interlocking seams.

Steel Roof Deck

a. Steel roof deck shall be 1.1/2" deep X 20 gage minimum painted, type "B" wide rib deck with interlocking side seams with the following properties:

	20 Gage
Minimum S (in^3/ft) =	0.237
Minimum I (in^4/ft) =	0.231

b. Minimum allowable deck diaphragm shear values shall be 1300 plf for a 7'-0" deck span.
c. Weld steel roof deck to supporting framing members with 3/4" diameter puddle welds at the following spacings (Closer spacing may be used to develop minimum shear requirements.):
   i. 6" o.c. to all supports perpendicular to deck corrugations (7 welds per 36" sheet).
   ii. 6" o.c. to all supports parallel to deck corrugations.
d. Hilti or Pneutek power driven fasteners are acceptable as an alternative to welds provided the connection meets the diaphragm shear capacity given above. For Hilti call 800-879-8000 extension 6337 for connection information comparison. For Pneutek, call 800-431-8665. If Hilti or Pneutek power driven fasteners are used, the contractor shall submit Hilti's / Pneutek calculations to the Architect/Engineer for review. Also if Hilti of Pneutek power driven fasteners are used, a Hilti / Pneutek representative shall be present before the decking is installed to make sure the installer is properly trained in using the equipment. The Hilti / Pneutek representative shall also make a site visit the day after deck has been started to be installed to verify the power driven fasteners are being installed correctly.
e. Attach interlocking seams with one of the following:
   i. 1 ½" long top seam welds at 8" o.c. at Stage roof, 24 o.c. maximum elsewhere.
   ii. Verco PunchLok II System at 8" o.c. at Stage roof, 24" o.c. maximum elsewhere.
   iii. ASC Delta Grip System at 12" o.c. at Stage roof, 36" o.c maximum elsewhere.
   iv. CSI Inter-Knek System at 12" o.c. at Stage roof, 36" o.c maximum elsewhere.

   Closer spacing may be used to develop minimum shear requirements. A standard button punch can **not** be used in place of Verco PunchLok, DeltaGrip or CSI Inter-Knek
f. Provide a 2-inch minimum bearing and a 4-inch lap at the splice points.

LEGEND OF MARKS AND ABBREVIATIONS

AB	ANCHOR BOLT(S)	k	KIP(S) = 1000 POUNDS
ABV	ABOVE	KLF	KIPS PER LINEAL FOOT
ALT	ALTERNATE	KSF	KIPS PER SQUARE FOOT
APPROX	APPROXIMATE		
ARCH	ARCHITECT(URAL)	LBS	POUNDS
		LF	LINEAL FOOT
BLDG	BUILDING	LLH	LONG LEG HORIZONTAL
BLW	BELOW	LLV	LONG LEG VERTICAL
BM	BEAM	LSH	LONG SIDE HORIZONTAL
BOT	BOTTOM	LSV	LONG SIDE VERTICAL
BRG	BEARING		
BTWN	BETWEEN		
		MAS	MASONRY
CC.	CENTER-TO CENTER	MAX	MAXIMUM
CGS	CENTER OF GROSS STEEL	MCJ	MASONRY CONTROL JOINT
C.J.	CONST/CONTROL JOINT	MECH	MECHANICAL
CJP	COMPLETE JOINT PENETRATION	MF-X	MOMENT FRAME
	GROOVE WELD (FULL PEN WELD)	MFR	MANUFACTURER
CMU	CONCRETE MASONRY UNIT	MIN	MINIMUM
COL	COLUMN	MISC	MISCELLANEOUS
CONC	CONCRETE	ML-x	MASONRY LINTEL
CONST	CONSTRUCTION	MP-x	MASONRY PIER
CP-x	CONCRETE PIER	MW-x	MASONRY WALL
CRW-x	CONCRETE RETAINING WALL		
CTR	CENTER	NIC	NOT IN CONTRACT
CW-x	CONCRETE WALL	NTS	NOT TO SCALE
DB	DECK BEARING	O.C.	ON CENTER
DBA	DEFORMED BAR ANCHOR	O.F.	OUTSIDE FACE
DBE	DECK BEARING ELEVATION	OPNG	OPENING
DBL	DOUBLE	OPP	OPPOSITE
DET	DETAIL	OWSJ	OPEN WEB STEEL JOISTS
DIA	DIAMETER		
DIM	DIMENSION	PAF	POWDER-ACTUATED FASTENER
DN	DOWN	PCF	POUNDS PER CUBIC FOOT
DWG	DRAWING	PL	PLATE
DWL	DOWEL	PLF	POUNDS PER LINEAL FOOT
		PSF	POUNDS PER SQUARE FOOT
EA	EACH	PSI	POUNDS PER SQUARE INCH
E.F.	EACH FACE	PT	POINT
E.J.	EXPANSION JOINT		
ELEC	ELECTRICAL	REINF	REINFORCING
ELEV	ELEVATION	REQD	REQUIRED
E.O.D.	EDGE OF DECK	R.D.	ROOF DRAIN
E.O.S.	EDGE OF SLAB	RTU	ROOF TOP UNITS
EQUIP	EQUIPMENT		
EQ	EQUAL	SBP-x	STEEL BASE PLATE MARK
E.W.	EACH WAY	SC-x	STEEL COLUMN MARK
EXT	EXTERIOR	SCP-x	STEEL CAP PLATE MARK
		SHT	SHEET
FC-x	CONTINUOUS FOOTING MARK	SI	SPECIAL INSPECTION
F.D.	FLOOR DRAIN	SIM	SIMILAR
FDN	FOUNDATION	SMU	SUSPENDED MECHANICAL UNITS
F.F.	FINISHED FLOOR	SOG	SLAB-ON-GRADE
FR-x	RECTANGULAR FOOTING	SQ	SQUARE
FS-x	SQUARE FOOTING MARK	STAG	STAGGERED
FT	FOOT	STD	STANDARD
FTG	FOOTING	STL	STEEL
FTS-x	THICKEN SLAB MARK	STR	STRUCTURAL
		STS	SELF TAPPING SCREWS
GA	GAUGE		
GALV	GALVANIZED	T&B	TOP AND BOTTOM
GSN	GENERAL STRUCTURAL NOTES	TEMP	TEMPERATURE
		THDS	THREADS
HB	HORIZONTAL BRIDGING	T.O.	TOP OF
HORIZ	HORIZONTAL	TOC	TOP OF CONCRETE
HSA	HEADED STUD ANCHOR	TOD	TOP OF DECK
HT	HEIGHT	TOF	TOP OF FOOTING
		TOS	TOP OF STEEL
		TOW	TOP OF WALL
ICC	INTERNATIONAL CODE COUNCIL	TYP	TYPICAL
IBC	INTERNATIONAL BUILDING CODE		
I.F.	INSIDE FACE	UNO	UNLESS NOTED OTHERWISE
IN.	INCH		
INT	INTERIOR	VERT	VERTICAL
JT	JOINT	W/	WITH
JST	JOIST	WT	WALL THICKNESS
		WWF	WELDED WIRE FABRIC
		WWM	WELDED WIRE MESH

SPECIAL INSPECTION SCHEDULES

STATEMENT OF SPECIAL INSPECTION AND QUALITY ASSURANCE

Special inspection and quality assurance, as required by section 1704 and 1705 of the 2015 IBC, shall be provided by an independent agency employed by the owner unless waived by the building official.	
The names and credentials of the Special Inspectors to be used shall be submitted to the Building Official for approval.	
Responsibilities of the Special Inspector	
	Special Inspector shall review all work listed in the special inspection schedules herein for conformance with the approved construction plans, specifications and 2015 IBC.
	All testing and inspection reports shall be sent within 24 hours of the test to the architect, engineer, building official and contractor for review. All items not in compliance shall be brought to the immediate attention of the contractor for correction, and if uncorrected, to the architect, engineer and building official.
	Once corrections have been made by the contractor, the special inspector shall submit a final signed report to the building official stating that the work requiring special inspection was, to the best of the special inspector's knowledge, in conformance with the approved construction plans, specifications and 2015 IBC.
Responsibilities of the Contractor	
	The contractor shall submit a written statement of responsibility to the owner and the building official prior to the commencement of work in accordance with 2015 IBC section 1704.4. This statement shall indicate that the contractor will coordinate and cooperate with the required inspections contained herein,
	The contractor shall notify the designated special inspector that work is ready for inspection at least 24 hours before said inspection is required.
	All work requiring special inspection shall remain open and accessible until it has been observed by the special inspector and deemed acceptable through inspection report.
	Special inspection during fabrication is not required if the fabricator is registered and approved to perform such work without special inspection.

SOILS CONSTRUCTION INSPECTIONS

Soils (2015 IBC Section 1705.6)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Site Preparation	-	X	Verify that the site has been prepared in accordance with the soils report prior to placement of prepared fill.
Fill Material	X	-	Verify that the material being used, the maximum lift thickness and the in-place dry density of the compacted fill material comply with the soils report during placement and compaction of the fill material during placement and compaction.
Continuous Footing Backfill: at least one test for each 40 linear feet or less of wall length, but no fewer than 2 tests.	-	X	At each compacted backfill layer.
Spot Footing Backfill: Minimum of one compaction test for each lift for each spot footing.	-	X	At each compacted backfill layer.
See specifications for further requirements.	-	-	

CONCRETE CONSTRUCTION INSPECTIONS

Concrete (2015 IBC Section 1705.3 and Section 1705.12.1) The following concrete elements require special inspection:

All concrete footings, all concrete walls, including foundation walls.

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Protection of concrete during cold and hot weather	-	X	
Verify materials used including use of the required mix design	-	X	Verify mix design meets strength and exposure requirements listed on General Structural Notes
Formwork	-	X	Verify shape, location and member dimensions
Bolts installed in concrete	X	-	Inspection of anchors or embeds cast in concrete is required when allowable loads have been increased or where strength design is used. Prior to and during concrete placement.
Embeds and Inserts installed in concrete	X	-	Prior to and during concrete placement.
Concrete reinforcing steel placement	-	X	Verify that reinforcing is of specified type, grade and size; that it is free of oil, dirt and rust; that it is located and spaced properly; that hooks, bends, ties, stirrups and supplemental reinforcement are placed correctly; that lap lengths, stagger and offsets are provided; and that all mechanical connections are installed per the manufacturer's instructions and/or evaluation report.
Concrete placement and samples	X	-	Cylinders, slump, temperature and air-entrainment shall be done for every 150 cubic yards or each day's production if the day's production is less than 150 cubic yards nor less than once for each 5000 sq. ft of surface area for slabs and walls.
See specifications for further concrete testing requirements.	-	-	

STEEL BOLTED CONNECTIONS INSPECTIONS

High Strength bolted connections (2015 IBC section 1705.2.1, section 1705.11.1 and section 1705.12.2 and AISC 360-10 Chapter N and AISC 341-10 Chapter J)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Inspection Tasks Prior to Bolting (Operations need not be delayed pending these inspections.)			
Manufacturer's certifications available for fastener materials	-	X	
Fasteners	-	X	Marked in accordance with ASTM requirements
Proper fasteners selected for the joint detail	-	X	Including grade, type, bolt length if threads are to be excluded from shear plane.
Proper bolting procedure selected for joint detail	-	X	
Connecting elements	-	X	Including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements
Proper storage	-	X	Storage provided for bolts, nuts, washers and other fastener components
Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used	X	-	Not required if only snug-tight joints are specified per [Section N5.6(1) of AISC 360-10])
Inspection Tasks During Bolting (Operations need not be delayed pending these inspections.)			
Fastener assemblies, of suitable condition,	-	X	Verify that fasteners placed in all holes and washers (if required) are positioned as required.
Joint	-	X	Verify that joint brought to the snug-tight condition (min) unless noted otherwise.
Fastener component	-	X	Verify that fastener component not turned by the wrench prevented from rotating
Pretensioned Fasteners	-	X	Verify that pretensioned fasteners are pretensioned in accordance with the RCSC *Specification*, progressing systematically from the most rigid point toward the free edges (Not required if only snug-tight joints are specified per [Section N5.6(1) of AISC 360-10); Not required for pretensioned joints using turn-of-the-nut method with match-marking, direct-tension-indicators or twist-off type tension)
Inspection Tasks After Bolting			
Document acceptance or rejection of bolted connection	X	-	

STEEL WELDED CONNECTIONS INSPECTIONS

Structural Welding (2015 IBC section 1705.2 and section 1705.11.1 and section 1705.12.2 and AISC 360-10 Chapter N and AISC 341-10 Chapter J)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Inspection Tasks Prior to Welding (Operations need not be delayed pending these inspections.)			
Welding procedures specifications and manufacturer certifications for welding consumables shall be available	X	-	
Material identification (type/grade)	-	X	Inspection tasks required to be performed for each welded joint or member.
Welder identification system	-	X	Verify there is a system in place to identify the welder who has welded a joint or member.
Fit-up of groove welds	-	X	Including joint geometry, joint preparation, dimensions, cleanliness, tacking and backing type and fit; to be performed for each welded joint or member.
Configuration and finish of access holes	-	X	Verify configuration and finish.
Fit-up of fillet welds	-	X	Including alignment, gaps at root, dimensions, cleanliness and tacking; to be performed for each welded joint or member.
Check welding equipment	-	X	Operations need not be delayed pending these inspections.
Inspection Tasks During Welding			
Use of qualified welders	-	X	
Control and handling of welding consumables	-	X	Including packaging and exposure control
Cracked tack welds	-	X	Verify no welding over cracked tack welds.
Environmental conditions	-	X	Including wind speed within limits and precipitation and temperature
WPS followed	-	X	including settings on welding equipment, travel speed, selected welding materials, shielding gas type/flow rate, preheat applied, interpass temperature (min./max.) maintained, proper position (F, V, H, OH)
Welding techniques	-	X	Including interpass and final cleaning, each pass within profile limitations, each pass meets quality requirements
Single-pass fillet welds less than 3/8"	-	X	
Single-pass fillet welds equal or greater than 3/8"	X	-	
Multipass fillet welds	X	-	
Complete and partial penetration groove welds	X	-	
Inspection Tasks After Welding (Operations need not be delayed pending these inspections.)			
Welds cleaned	-	X	
Size, length and location of welds	X	-	Inspection tasks required to be performed for each welded joint or member.
Welds meet visual acceptance criteria such as: crack prohibition, weld/base-metal fusion, crater cross section, weld profiles, weld size, undercut and porosity	X	-	Inspection tasks required to be performed for each welded joint or member.
Arc strikes, k-area, backing removed and weld tabs removed (if required), repair activities	X	-	Inspection tasks required to be performed for each welded joint or member.
Document acceptance or rejection of welded joint or member	X	-	Inspection tasks required to be performed for each welded joint or member.

OPEN-WEB STEEL JOIST CONSTRUCTION INSPECTIONS

Installation of open-web steel joists (IBC 2015 Section 1705.2.3 and Table 1705-2-3)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
End connections – welding or bolted	-	X	Follow SJI references listed in section 2207.1
Bridging – horizontal and diagonal	-	-	
Standard bridging	-	X	Follow SJI references listed in section 2207.1
Bridging that differs from SJI specifications listed in section 2207.1	-	X	

MISCELLANEOUS STEEL CONSTRUCTION INSPECTIONS

Structural Welding of Metal Deck Construction (2015 IBC section 1705.2 and section 1705.11.1 and section 1705.12.2 and AISC 360-10 Chapter N and AISC 341-10 Chapter J)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Material verification of steel deck(s)	-	X	Indentification markings per applicable ASTM standard
Roof deck welding	-	X	Visual inspection is required to confirm that weld meets acceptance criteria of AWS D1.3. Welder qualifications should be verified

Mechanical attachments of Metal Deck Construction (2015 IBC Table 1705.1.1)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Metal roof decks.	-	X	Verify size and spacing of shot pins / screws for deck attachment to the supporting structure. Also verify spacing and size of seam attachments.

POST-INSTALLED ANCHOR INSPECTIONS

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Post Installed Anchors and Reinforcing Bars (2015 IBC Section 1705.1.1)			
Epoxy Anchors and Reinforcing Bars	-	X	Special inspection shall be performed per manufacturer's requirements and approved ICC-ES reports noted in POST-IINSTALLED ANCHOR section of the General Structural Notes prior to installation of epoxy and anchor rod.
Mechanical Anchors and Screw Anchors	-	X	Special inspection shall be provided per manufacturer's requirements and approved ICC-ES reports noted in POST-INSTALLED ANCHOR section of the General Structural Notes prior to installation of mechanical or screw anchor.

MASONRY CONSTRUCTION INSPECTIONS

Prior to Construction (2015 IBC section 1705.4 and ACI 530.1/ASCE 6/TMS 602 1.11)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Review material certificates, mix designs, test results and construction procedures	-	X	Verify that materials conform to the requirements of the approved construction documents. Mix design, test results, material certificates, and construction procedures should be submitted for review. Mortar mix designs shall conform to ASTM C 270 while grout shall conform to ASTM C 476. Material certificates shall be provided for the following: reinforcement; anchors, ties, fasteners, and metal accessories; masonry units; mortar and grout materials. Construction procedures for cold-weather or hotweather construction shall be reviewed.

As Construction begins (2015 IBC section 1705.4 and ACI 530.1/ASCE 6/TMS 602 Table 4)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Proportions of site-prepared mortar, construction of mortar joints,	-	X	
Location of reinforcement and connectors	-	X	
Size and location of structural elements	-	X	
Proportions of site-prepared mortar, construction of mortar joints,	-	X	

Prior to grouting and during construction - structural masonry shall have Level B special inspection (2015 IBC section 1705.4 and ACI 530.1/ASCE 6/TMS 602 Table 4)

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
			Verified for every 5,000 square feet of masonry constructed for periodic special inspection items
Proportions of site-prepared mortar, construction of mortar joints,	-	X	
Size and location of structural elements	-	X	
Grade, type and size of reinforcement, connectors and anchors	-	X	
Location and placement of reinforcement, connectors and anchors	-	X	
Verify grout space is clean prior to grouting	-	X	
Protection of masonry during cold and hot weather	-	X	
Grout placement	X	-	
Observe preparation of grout specimens, mortar specimens and/or prisms	-	X	The contractor has the option of using the "Prism Test Method" per ACI 530.1/ASCE 6/TMS 602 in lieu of the "Unit Strength Method."

NON-STRUCTURAL COMPONENT CONSTRUCTION INSPECTIONS

ITEM FOR VERIFICATION & INSPECTION	INSPECTION FREQUENCY		COMMENTS
	CONTINUOUS	PERIODIC	
Architectural Components located in Seismic Design Categories D, E and F (2015 IBC Sections 1705.12.5 and 1705.12.7)			
Erection and fastening of interior and exterior nonbearing walls	-	X	Verify appropriate materials, fasteners and attachment at commencement of work and at completion. (Not required if <30 feet or for interior walls < 15 psf.)
Mechanical and Electrical Components located in Seismic Design Categories D, E and F (2015 IBC Sections 1705.12.4 and 1705.12.6)			
Anchorage of emergency or standby power systems	-	X	Verify that anchorage complies with approved construction documents.
Designated seismic systems	-	X	Verify that manufacturer's certificate of compliance conforms to the requirements of Section 13.2 of ASCE 7-10. Verify that the label, anchorage or mounting conforms to the manufacturer's certificate of compliance.

STRUCTURAL OBSERVATION PROGRAM

If structural observations are required, they shall be done by the Engineer of Record or an approved subordinate at the stages of construction listed in the Construction Notification Phases section of these notes. At the conclusion of the project, the designated structural observer shall submit to the building official a written statement that the site visits have been made and identify any reported deficiencies that to the best of the structural observer's knowledge have not been resolved (See IBC 2015 1704.6).

STRUCTURAL OBSERVATION PROGRAM REQUIRED BY CODE:	YES	NO
		X

CONSTRUCTION MILESTONE SCHEDULE

CONTRACTOR TO NOTIFY ENGINEER AT THE FOLLOWING CONSTRUCTION PHASES:	
CONCRETE	
Footings, stem walls and piers	Prior to pouring concrete
STEEL	
Roof framing	After substantial portion of framing is erected
Roof deck	After welding/fastening and prior to roofing
MASONRY	
Masonry walls	Prior to pouring grout

DEFERRED SUBMITTALS

For the purpose of this section, deferred submittals are defined as per section 107.3.4.1 of the IBC 2015. Submittal documents for deferred submittal items shall be submitted to the engineer, architect and building official for their review for general conformance with the design of the building.

DEFERRED STRUCTURAL SUBMITTALS FOR THIS PROJECT ARE:

Open web steel joists (per IBC 2015 section 2207)

BRETT KARL GOODMAN
No. 7991694-2203
LICENSED PROFESSIONAL STRUCTURAL ENGINEER
STATE OF UTAH

12-1-2017

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project:
Barker Park Amphitheater

North Ogden, Utah

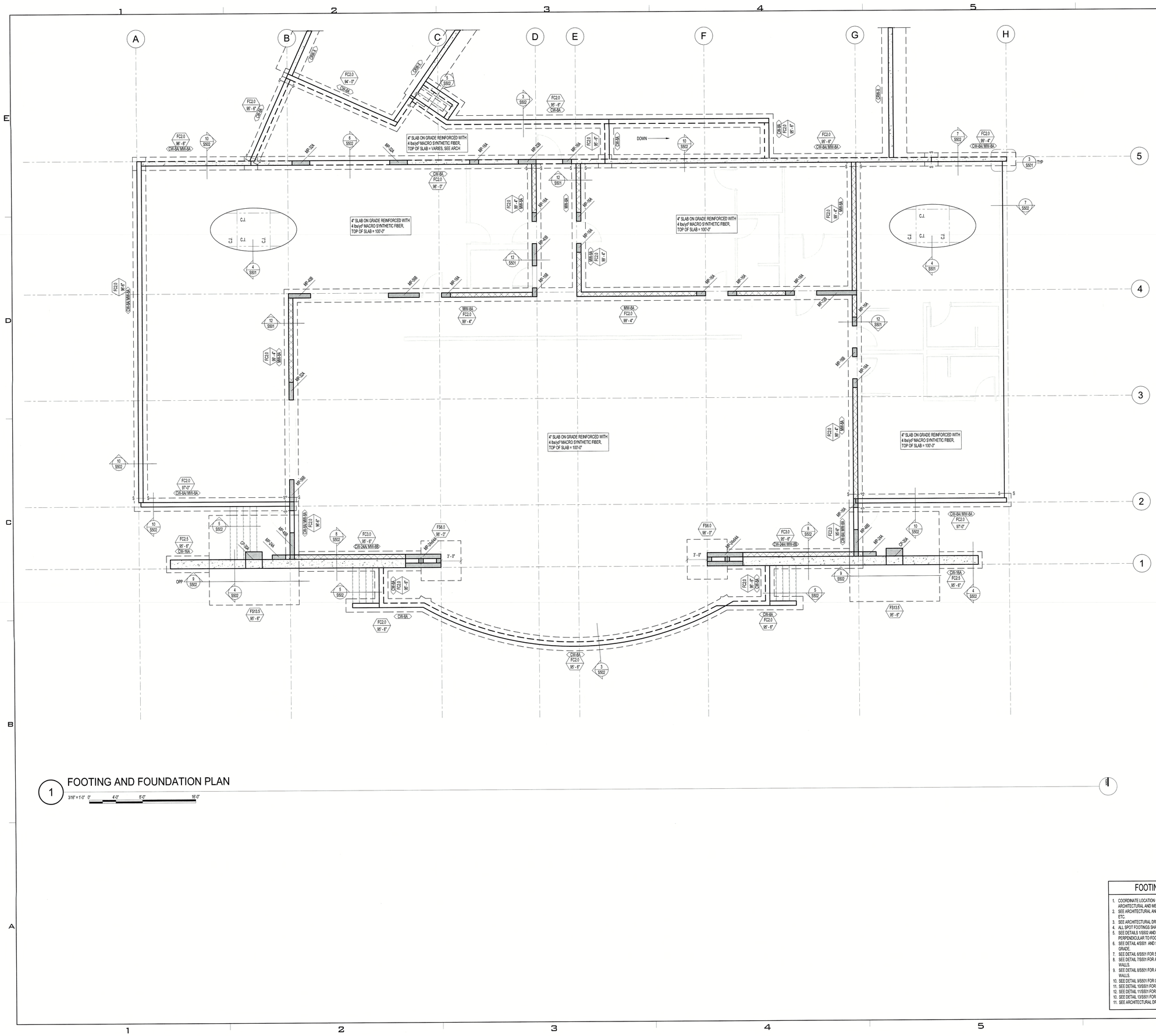
project#: 170955
date: 01 December 2017

revisions :

title:
SPECIAL INSPECTION SCHEDULES

sheet:
S002

Phase 1 Bid Package 2



1 FOOTING AND FOUNDATION PLAN

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

MARKS AND SYMBOLS LEGEND

- SECTION MARK / SHEET NUMBER
- FOOTING DESIGNATION / TOP OF FOOTING ELEVATION
- S——S INDICATES FOOTING STEP, SEE DETAIL 1/S501
- INDICATES CONCRETE WALL. DASHED WALLS STOP AT DECK
- INDICATES MASONRY WALL. DASHED WALLS STOP AT DECK
- INDICATES STRUCTURAL WALL ABOVE.
- DEPRESS FOUNDATION WALL AND POUR SLAB OVER. SEE DETAIL 6/S502
- INDICATES DEPRESSED SLAB, SEE ARCHITECTURAL PLANS.
- CRW-x INDICATES CONCRETE FOUNDATION WALL TYPE, SEE SCHEDULE ON SHEET S601
- MW-x INDICATES MASONRY WALL TYPE, SEE SCHEDULE ON SHEET S602
- CRW INDICATES CONCRETE RETAINING WALL TYPE, SEE SCHEDULE ON SHEET S601
- FCx.x INDICATES CONTINUOUS FOOTING. SEE SCHEDULE ON SHEET S601
- FSx.x INDICATES SPOT FOOTING. SEE SCHEDULE ON SHEET S601
- FTSx.x INDICATES THICKENED SLAB FOOTING. SEE SCHEDULE ON SHEET S601
- CP-x INDICATES CONCRETE PIER. SEE SCHEDULE ON SHEET S601

FOOTING AND FOUNDATION PLAN NOTES

1. COORDINATE LOCATION OF DEPRESSED SLABS, SLOPED SLABS, AND FLOOR DRAINS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. SEE ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR CONCRETE WORK AT DOORS, SIDEWALKS, ETC.
3. SEE ARCHITECTURAL DRAWINGS FOR CONTROL JOINT LOCATIONS.
4. ALL SPOT FOOTINGS SHALL BE CENTERED UNDER COLUMNS (U.N.O.).
5. SEE DETAILS 1/S502 AND 2/S502 FOR CONDITIONS WHERE BURIED PIPES RUN PARALLEL AND PERPENDICULAR TO FOOTINGS.
6. SEE DETAIL 4/S501 AND 5/S501 FOR TYPICAL CONTROL/CONSTRUCTION JOINTS IN CONCRETE SLAB ON GRADE.
7. SEE DETAIL 8/S501 FOR SLAB REINFORCING WHERE CONTROL JOINTS ARE DISCONTINUOUS.
8. SEE DETAIL 7/S501 FOR ADDITIONAL REINFORCING AROUND MISCELLANEOUS OPENINGS IN CONCRETE WALLS.
9. SEE DETAIL 8/S501 FOR ADDITIONAL REINFORCING AROUND MISCELLANEOUS OPENINGS IN MASONRY WALLS.
10. SEE DETAIL 9/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
11. SEE DETAIL 10/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
12. SEE DETAIL 11/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
10. SEE DETAIL 13/S501 FOR ANCHORAGE OF HOUSEKEEPING PADS.
11. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO ALL STEEL COLUMNS.

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12-1-2017

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Barker Park
Amphitheater

North Ogden, Utah

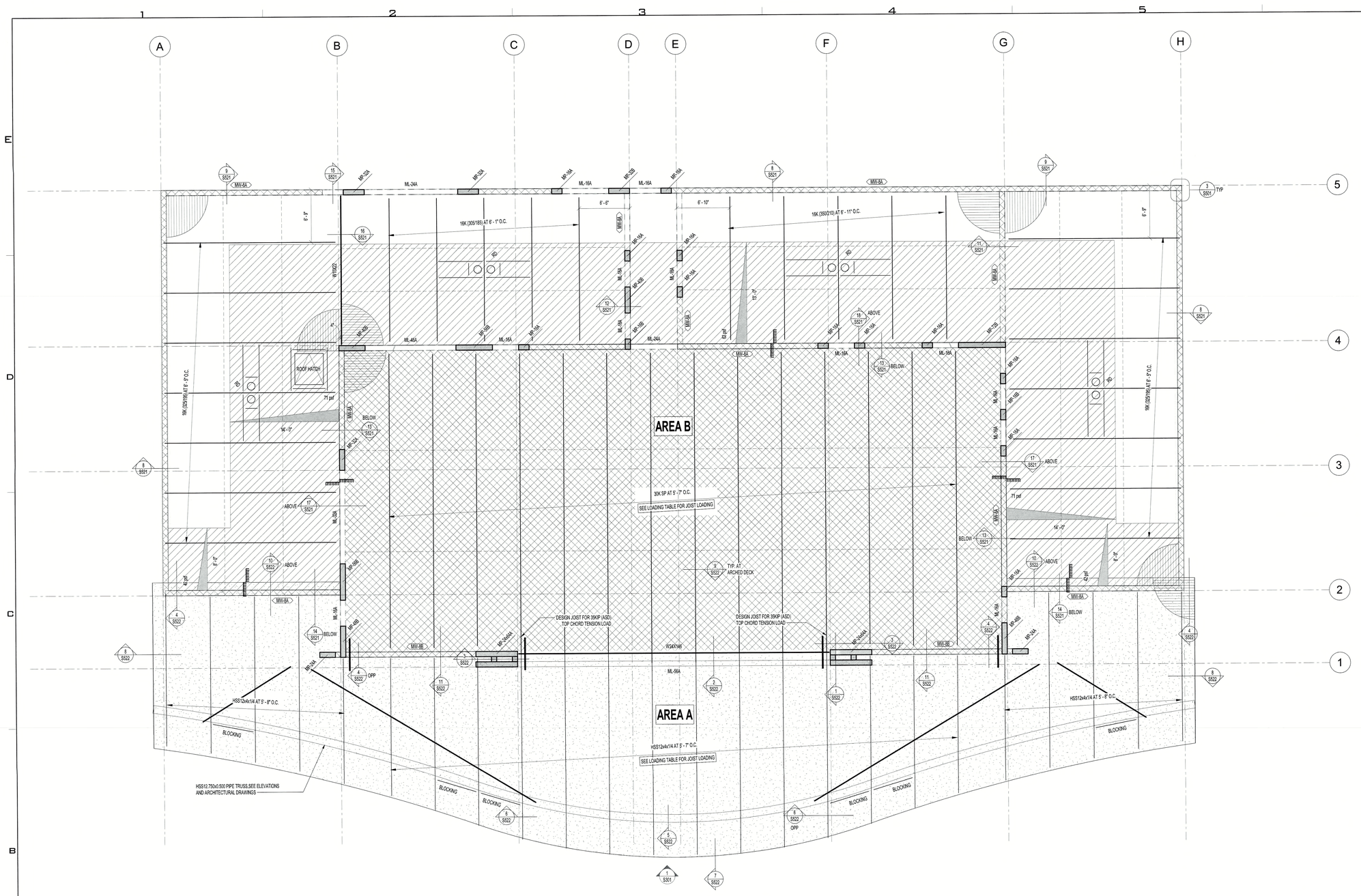
project#: 170955
date: 01 December 2017

revisions :

title:
FOOTING AND FOUNDATION PLAN

sheet:
S101

Phase 1 Bid Package 2



1 ROOF FRAMING PLAN

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

'AREA A' LOADING			
LOAD CASE	DEAD LOAD	SNOW LOAD	STAGE LIVE LOAD
CASE 1:	20 psf	36 psf	20 psf
CASE 2:	20 psf	0 psf	40 psf
CASE 3:	20 psf	0 psf	20 psf WITH (2) 5000lbs POINT LOADS ON TRUSS BETWEEN GRIDS C AND F

'AREA B' LOADING			
LOAD CASE	DEAD LOAD	SNOW LOAD	STAGE LIVE LOAD
CASE 1:	20 psf	36 psf	20 psf
CASE 2:	20 psf	0 psf	56 psf
CASE 3:	20 psf	0 psf	(1) 5000lb POINT LOAD AT ANY POINT ALONG JOIST

SNOW DRIFT LEGEND

INDICATES AREA OF SNOW DRIFT. JOIST SUPPLIER TO ADD SNOW DRIFT LOAD TO JOISTS AND GIRDERS

xx psf / x psf / x'-x" — INDICATES ADDITIONAL LOADING DUE TO SNOW DRIFT (SEE SNOW DRIFT NOTE)
xx psf INDICATES LOAD AT HIGH POINT.
x'-x" INDICATES LENGTH OF DRIFT.

xx psf / x psf / x'-x" — INDICATES ADDITIONAL LOADING DUE TO SNOW DRIFT (SEE SNOW DRIFT NOTE)
xx psf INDICATES LOAD AT HIGH POINT.
x psf INDICATES LOAD AT LOW POINT
x'-x" INDICATES LENGTH OF DRIFT
SEE PLAN FOR SNOW DRIFT LOADING.

SNOW DRIFT NOTE: THESE LOADS ARE IN ADDITION TO THE JOIST UNIFORM AND POINT LOADS SHOWN ON PLANS. JOIST SUPPLIER TO ADD SNOW DRIFT LOAD TO BOTH JOISTS AND GIRDERS

MARKS AND SYMBOLS LEGEND

- SECTION MARK / SHEET NUMBER
- INDICATES MASONRY WALL. DASHED WALLS STOP AT DECK
- MW-x — INDICATES MASONRY WALL TYPE. SEE SCHEDULE ON SHEET S602
- INDICATES METAL ROOF DECK. SEE GENERAL STRUCTURAL NOTES ON SHEET(S) S001
- _K ("/"/") — INDICATES K-SERIES JOIST WITH ALLOWABLE TOTAL LOAD / ALLOWABLE LIVE (SNOW) LOAD
- _LH ("/"/") — INDICATES LH-SERIES JOIST WITH ALLOWABLE TOTAL LOAD / ALLOWABLE LIVE (SNOW) LOAD
- MP-x — INDICATES MASONRY PIER TYPE. SEE SCHEDULE ON SHEET S602
- ML-x — INDICATES MASONRY LINTEL TYPE. SEE SCHEDULE ON SHEET S602
- R.D. — INDICATES ROOF DRAIN, SEE DETAIL 2/S521

ROOF FRAMING PLAN NOTES

1. VERIFY ALL ROOF OPENINGS FOR MECHANICAL SHAFTS, DRAINS, ETC. WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
2. JOIST SUPPLIER SHALL DESIGN ALL ROOF JOIST BEARING ENDS AT MASONRY WALLS TO TRANSFER 2000# (ALLOWABLE) AXIAL LOAD THROUGH JOIST BEARING SHOE.
3. ALL JOISTS SHALL HAVE 5" DEEP BEARING ENDS (UNO).
4. ALL ROOF OPENINGS GREATER THAN, OR EQUAL TO, 12" x 12" SHALL BE FRAMED AS INDICATED IN DETAILS 1/S521 AND 2/S521. FOR OPENINGS WHICH CUT LESS THAN TWO DECK FLUTES, SEE DETAIL 3/S521.
5. SEE DETAIL 4/S521 WHEN CONCENTRATED LOADS ARE LOCATED MORE THAN 6" FROM JOIST OR JOIST GIRDER PANEL POINT.
6. SEE DETAIL 9/S521 WHEN MECHANICAL UNITS ARE HUNG BELOW JOISTS.
7. VERIFY SIZE, WEIGHT, AND LOCATION OF ALL ROOF TOP MECHANICAL UNITS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. SEE DETAIL 6/S521 FOR STEEL FRAMES AT ALL ROOF TOP EQUIPMENT. COORDINATE OPENINGS WITH MECHANICAL, ELECTRICAL, AND GENERAL CONTRACTORS.
8. LOCATE MISCELLANEOUS MECHANICAL OPENINGS BETWEEN JOISTS, NOT UNDERNEATH THEM.
9. OPEN WEB STEEL JOISTS AND JOIST GIRDERS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT THE MECHANICAL AND LATERAL LOADS SHOWN ON THE ROOF FRAMING PLANS IN ADDITION TO THE UNIFORM AND POINT LOADS SHOWN.
10. JOIST BRIDGING SHOWN ON PLANS IS FOR REPRESENTATION ONLY. ACTUAL SIZE, QUANTITY, AND LOCATION WILL BE DETERMINED BY THE JOIST SUPPLIER PER SJI REQUIREMENTS. ALL BRIDGING AND BRIDGING ANCHORS NEED TO BE IN PLACE BEFORE APPLYING ANY LOADS. WHERE SKYLIGHT OR MECHANICAL UNITS/DUCTS INTERRUPT HORIZONTAL BRIDGING, PROVIDE CROSS BRIDGING AT JOIST SPACES ON EACH SIDE OF THE OPENING. WHERE DIAGONAL BRIDGING CONFLICTS WITH MECHANICAL DUCTS, REMOVE DIAGONAL BRIDGING AND REPLACE WITH HORIZONTAL BRIDGING AFTER ROOF DECK IS IN PLACE.
11. JOIST DESIGNER SHALL DESIGN JOISTS AND GIRDERS AND SUPPLY ADDITIONAL BRIDGING AS REQUIRED FOR UPLIFT DUE TO WIND. ASSUME:
DEAD LOAD = 12psf WIND UPLIFT = 20psf (TOTAL-ALLOWABLE)
NO 1/3 STRESS INCREASE ALLOWED.
12. SEE DETAIL 8/S501 FOR ADDITIONAL REINFORCING AT MISCELLANEOUS OPENINGS IN MASONRY WALLS.
13. SEE DETAIL 9/S501 FOR CONDITION AT RECESSES IN MASONRY WALLS.
14. SEE DETAIL 10/S501 FOR TYPICAL CONTROL JOINTS IN MASONRY WALLS.
15. SEE DETAIL 11/S501 FOR TERMINATION OF HORIZONTAL REINFORCING IN MASONRY WALLS.
17. SEE ARCHITECTURAL PLANS FOR DIMENSIONS TO ALL STEEL COLUMNS.

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12-1-2017

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project:

Barker Park
Amphitheater

North Ogden, Utah

project#: 170955
date: 01 December 2017

revisions :

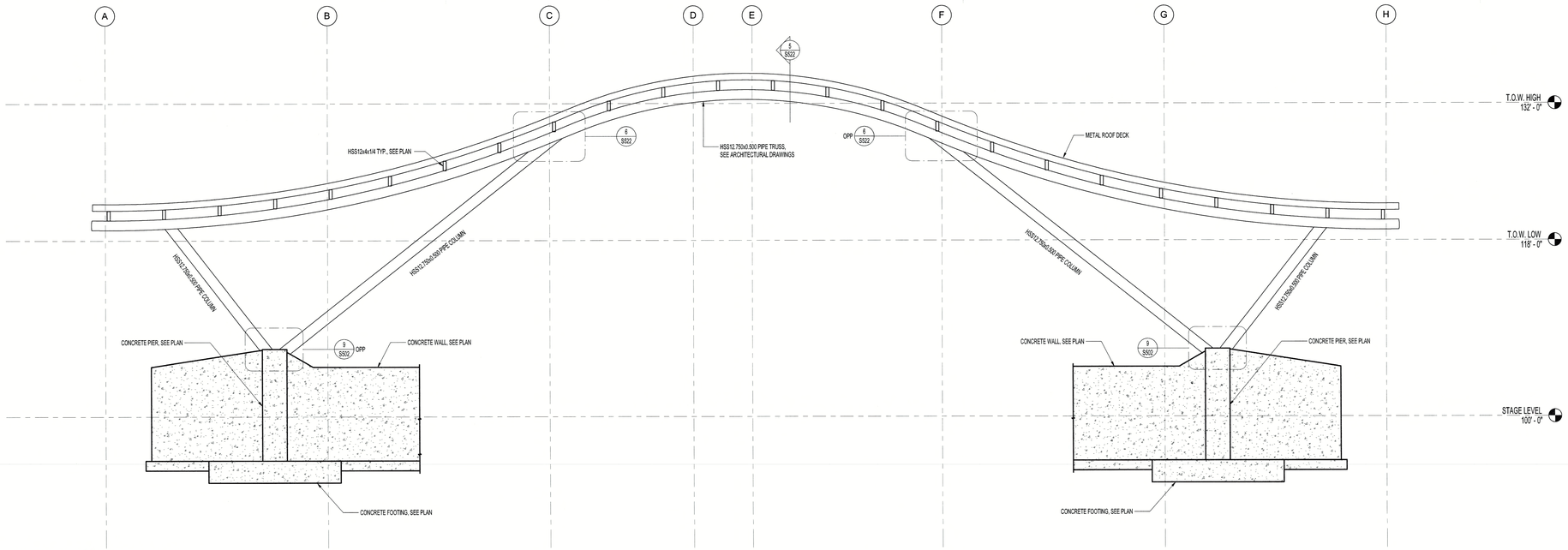
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ROOF FRAMING PLAN

sheet:

S121

Phase 1 Bid Package 2



1 STAGE TRUSS SECTION

3/16" = 1'-0" 0" 4'-0" 8'-0" 16'-0"

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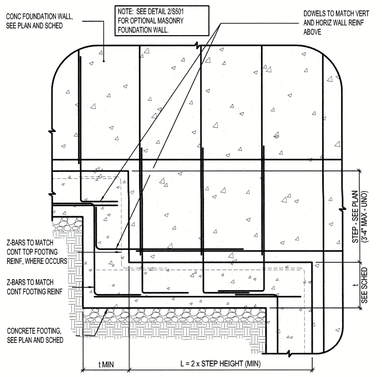
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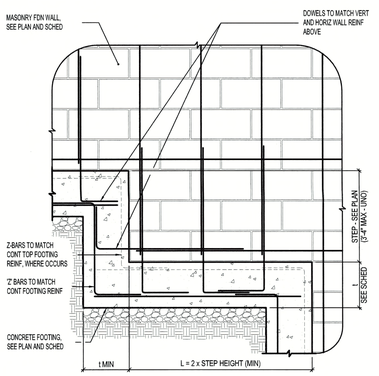
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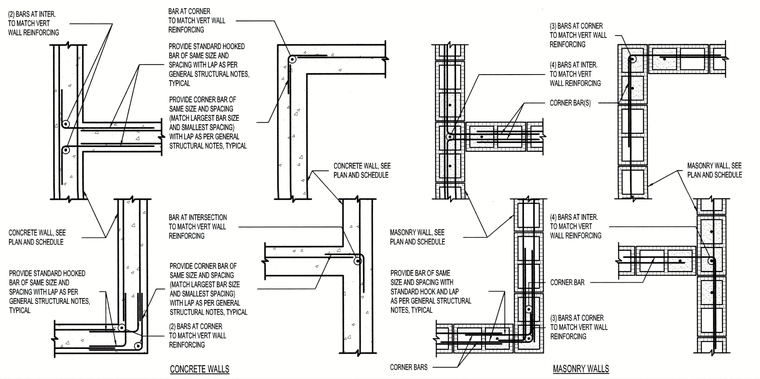
Phase 1 Bid Package 2



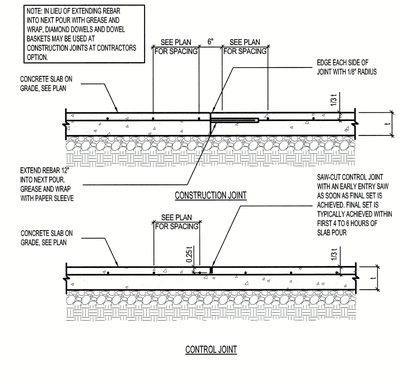
1 TYPICAL FOOTING STEP DETAIL — NO SCALE



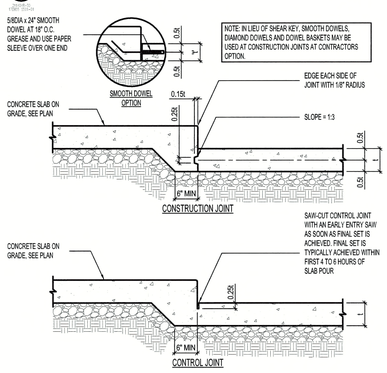
2 TYPICAL FOOTING STEP AT MASONRY FOUNDATION WALL [OPTIONAL] — NO SCALE



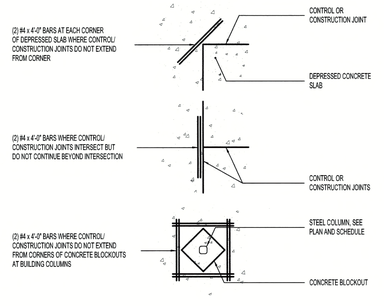
3 TYPICAL CORNER WALL REINFORCING [PLAN VIEW] — NO SCALE



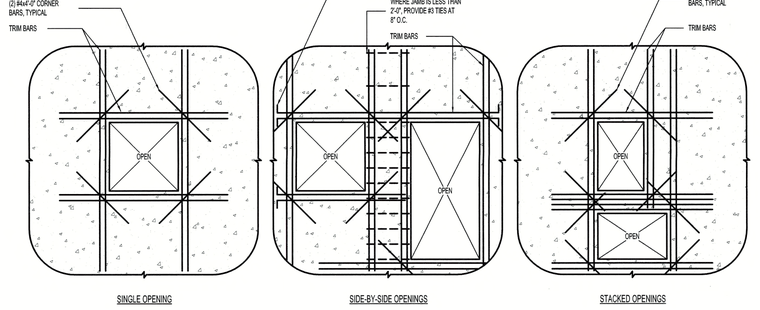
4 TYPICAL SLAB ON GRADE JOINT DETAILS — NO SCALE



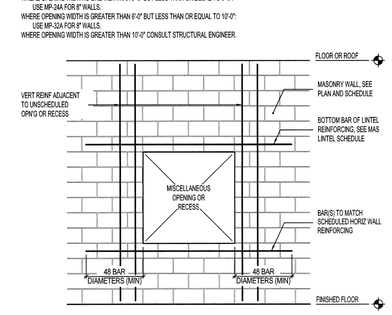
5 JOINT DETAILS AT SLAB DEPRESSIONS — NO SCALE



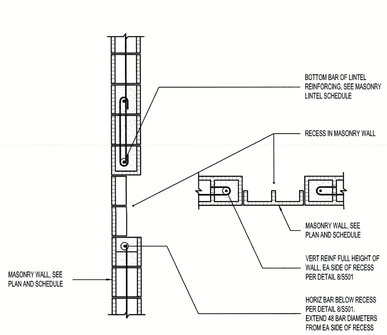
6 LOCATIONS REQUIRING ADDITIONAL SLAB REINFORCING [PLAN VIEW] — NO SCALE

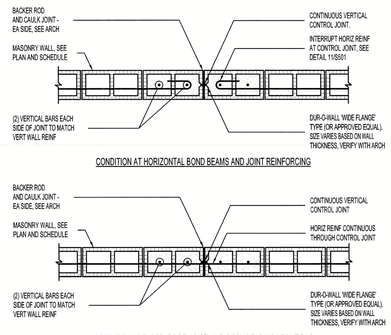


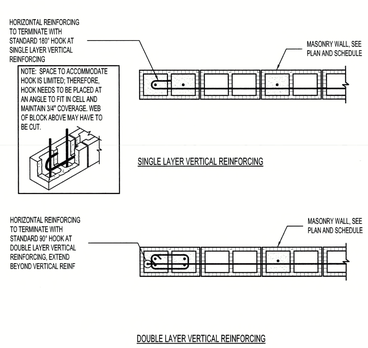
7 TYPICAL REINFORCING FOR MISCELLANEOUS OPENINGS LESS THAN 3'-0" IN CONCRETE WALLS — NO SCALE
WHERE MISCELLANEOUS OPENING WIDTH IS GREATER THAN 3'-0" WIDE, CONTACT STRUCTURAL ENGINEER.



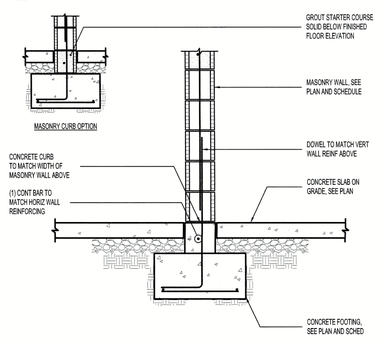
8 REINFORCING AT UNSCHEDULED MISCELLANEOUS OPENINGS OR RECESSES IN MASONRY WALLS — NO SCALE

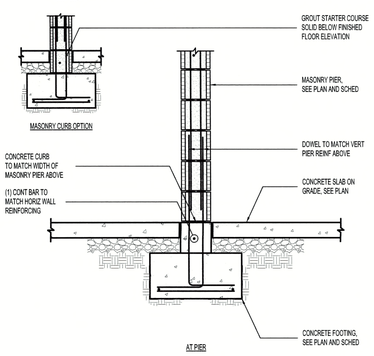




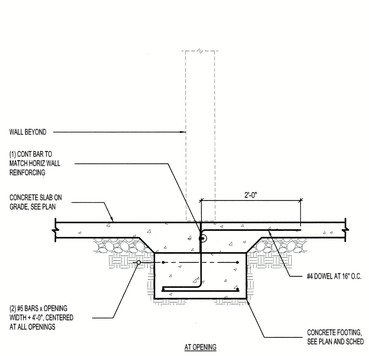


9 TYPICAL REINFORCING AT RECESS IN 8" OR 10" MASONRY WALLS — NO SCALE

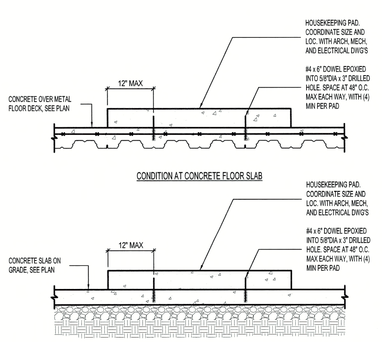




10 MASONRY CONTROL JOINT DETAIL AT 8" OR 10" MASONRY WALLS — NO SCALE



11 TERMINATION OF HORIZONTAL REINFORCING IN 8" OR 10" MASONRY WALL [PLAN VIEW] — NO SCALE



12 TYPICAL 8" OR 10" INTERIOR MASONRY WALL ON CONCRETE CURB — NO SCALE

13 HOUSEKEEPING PAD DETAIL — NO SCALE

LICENSED PROFESSIONAL STRUCTURAL ENGINEER
BRETT KARL GOODMAN
No. 7991094-2203
STATE OF UTAH
12-1-2017

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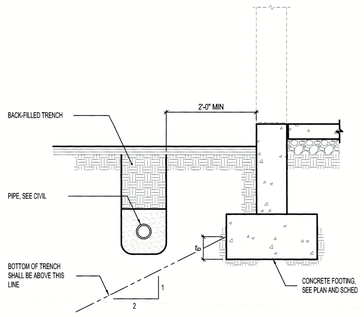
project#: 170955
date: 01 December 2017

revisions :

title:
FOUNDATION DETAILS

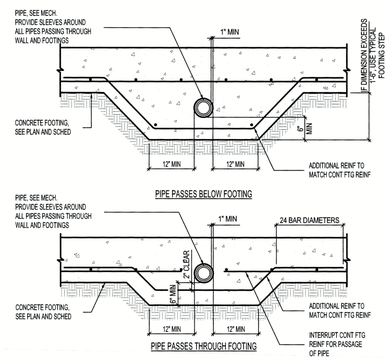
sheet:
S501

Phase 1 Bid Package 2



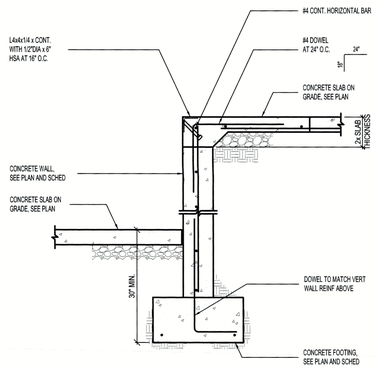
1 CONDITION AT PIPE PARALLEL TO CONCRETE FOOTING

NO SCALE



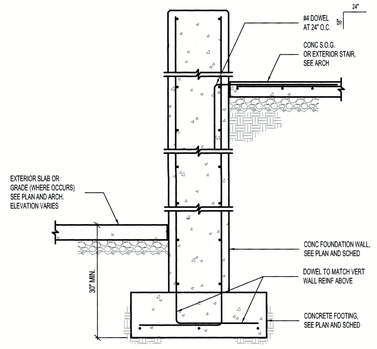
2 CONDITIONS AT PIPE PERPENDICULAR TO FOOTING

NO SCALE



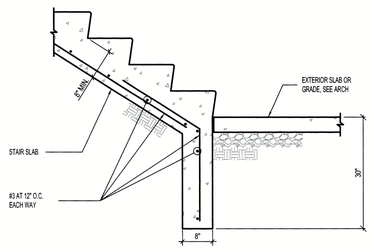
3 OFFSET CONCRETE FOUNDATION WALL

NO SCALE



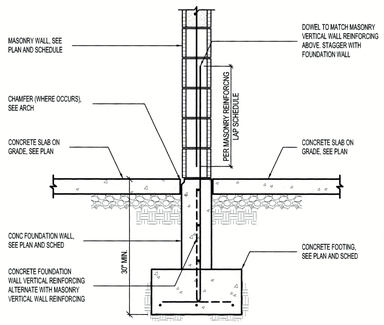
4 CONCRETE STAIR/LOADING RAMP

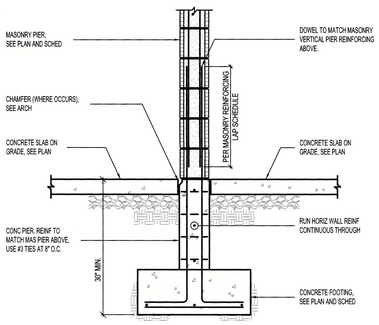
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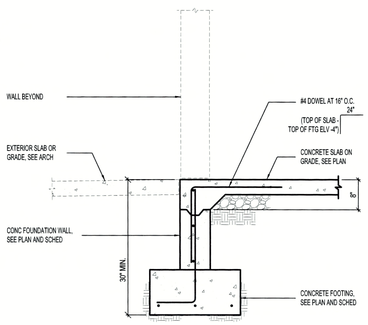


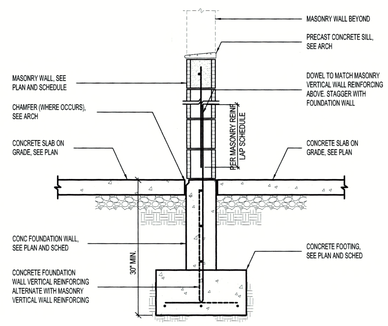
5 STAIR DETAIL

NO SCALE



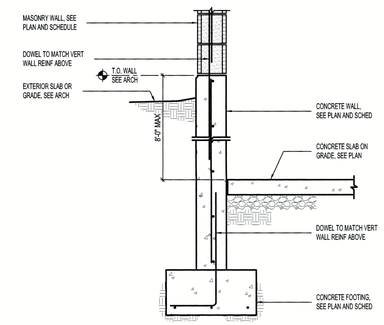






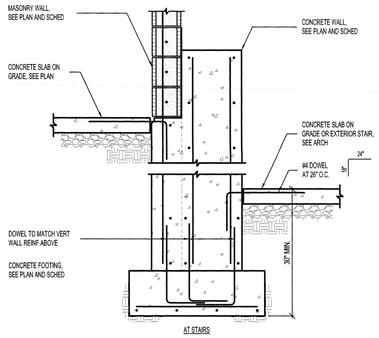
6 TYPICAL FOUNDATION WALL DETAIL AT EXTERIOR MASONRY WALL

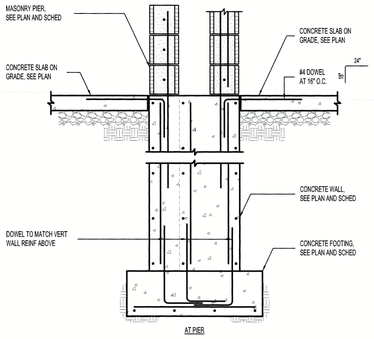
NO SCALE



7 CONCRETE FOOTING/WALL AT MASONRY WALL

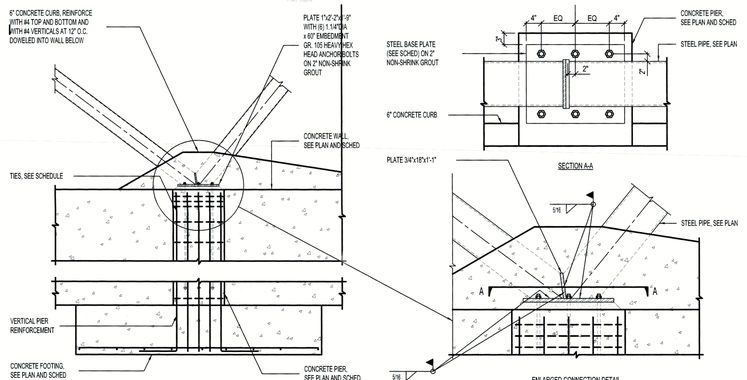
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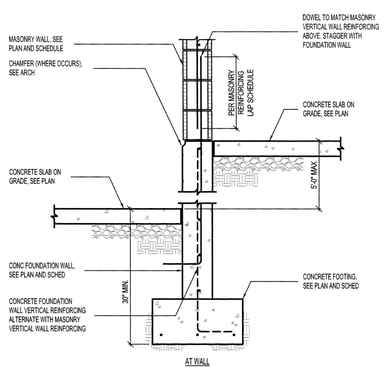
8 TYPICAL PIER DETAIL

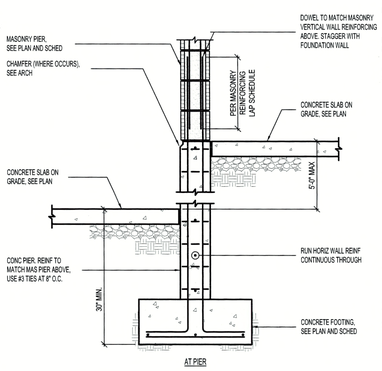
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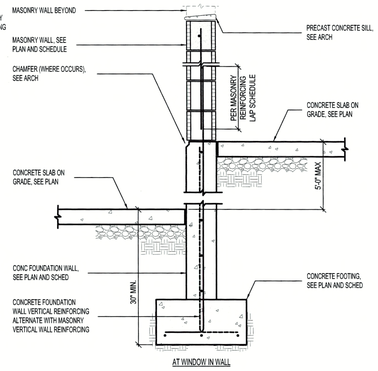


9 TRUSS BASE DETAIL

NO SCALE







10 TYPICAL FOUNDATION WALL DETAIL AT EXTERIOR MASONRY WALL

NO SCALE

BRETT KARL GOODMAN
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LICENSED PROFESSIONAL STRUCTURAL ENGINEER
STATE OF UTAH

12-1-2017

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project:

Barker Park Amphitheater

North Ogden, Utah

project#: 170955
date: 01 December 2017

revisions :

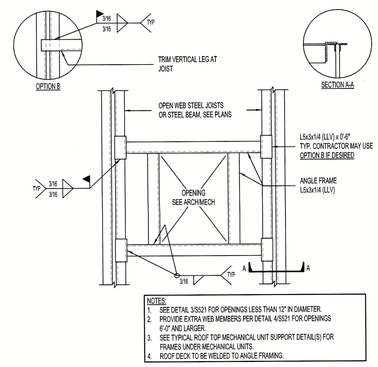
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FOUNDATION DETAILS

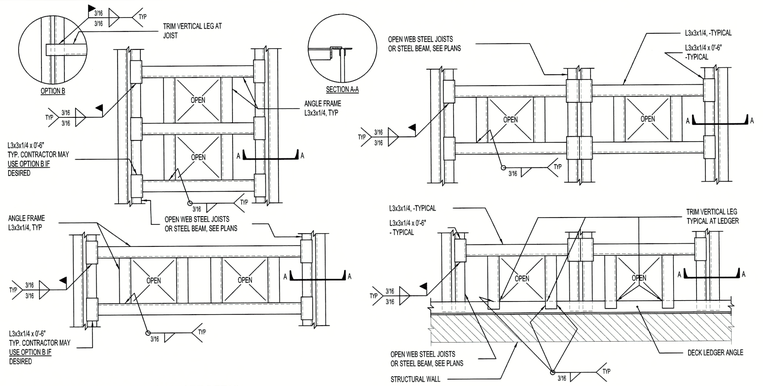
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S502

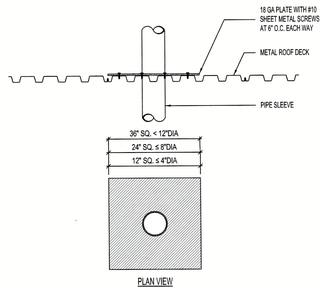
Phase 1 Bid Package 2



1 TYPICAL ROOF OPENING DETAIL [PLAN VIEW] — NO SCALE

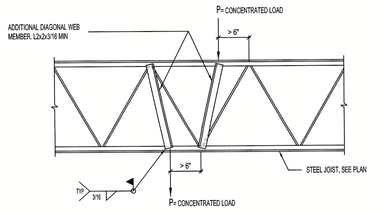


2 TYPICAL ROOF DRAIN SUPPORT DETAIL [PLAN VIEW] — NO SCALE



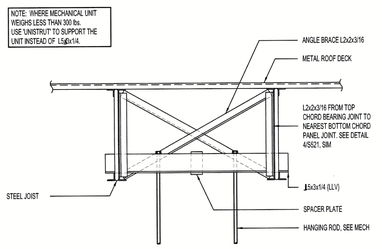
3 TYPICAL PIPE SLEEVE THROUGH ROOF DECK — NO SCALE

NOTE: WHERE CONCENTRATED LOADS ON OPEN WEB JOISTS ARE LOCATED MORE THAN 6" FROM THE PANEL WORK POINTS AT EITHER THE TOP OR BOTTOM CHORD, THE CONTRACTOR SHALL INSTALL ADDITIONAL DIAGONAL WEB MEMBERS AT THE LOCATION OF THE CONCENTRATED LOAD.



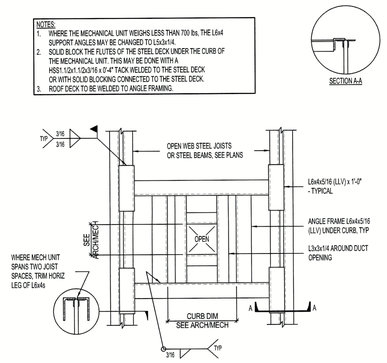
4 TYPICAL JOIST REINFORCING DETAIL FOR LOADS GREATER THAN 100 POUNDS — NO SCALE

NOTE: WHERE MECHANICAL UNIT WEIGHS LESS THAN 300 lbs USE UNISTRUT TO SUPPORT THE UNIT INSTEAD OF L5x3x1/4.

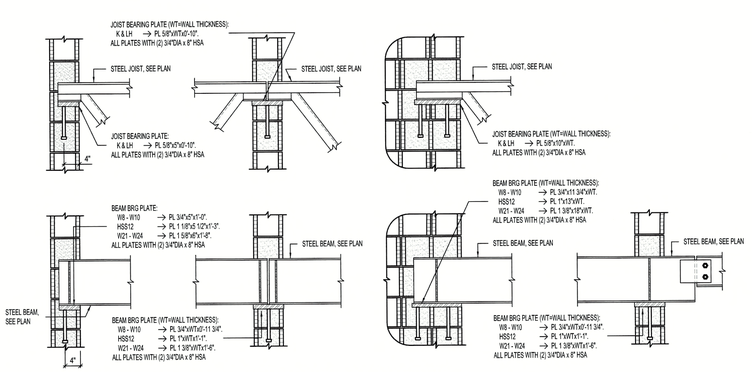


5 TYPICAL HANGING MECHANICAL UNIT SUPPORT DETAIL — NO SCALE

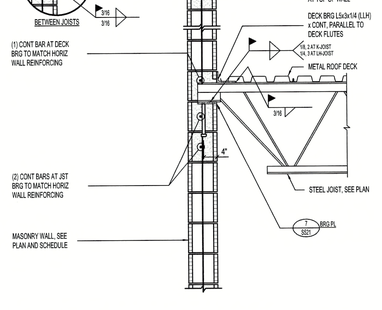
NOTES:
1. WHERE THE MECHANICAL UNIT WEIGHS LESS THAN 700 lbs, THE L6x4 SUPPORT ANGLES MAY BE CHANGED TO L5x3x1/4.
2. SOLID BLOCK THE FLUTES OF THE STEEL DECK UNDER THE CURB OF THE MECHANICAL UNIT. THIS MAY BE DONE WITH A HSS1.1/2x1.1/2x3/16 x 0'-4" TACK WELDED TO THE STEEL DECK OR WITH SOLID BLOCKING CONNECTED TO THE STEEL DECK.
3. ROOF DECK TO BE WELDED TO ANGLE FRAMING.



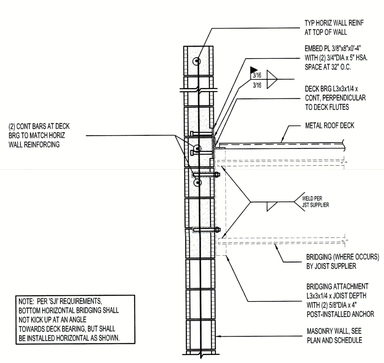
6 TYPICAL ROOF TOP MECHANICAL UNIT SUPPORT DETAIL [PLAN VIEW] — NO SCALE



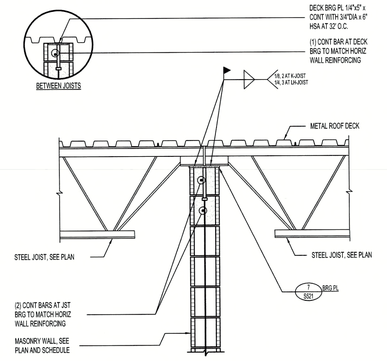
7 BEARING PLATE SCHEDULE FOR ROOF JOIST/BEAM BEARING AT MASONRY WALLS — NO SCALE



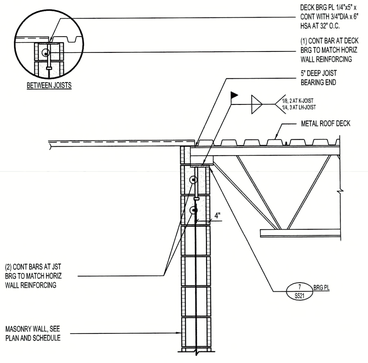
8 TYPICAL JOIST (5" BEARING ENDS) BEARING AT 8" MASONRY WALLS — NO SCALE



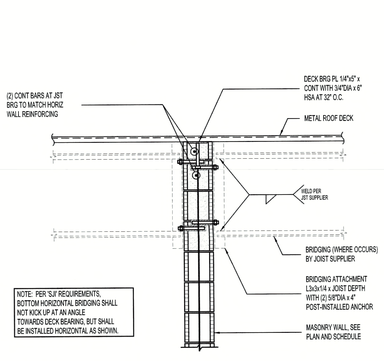
9 TYPICAL DECK BEARING WITH BRIDGING ATTACHMENT AT 8" MASONRY WALLS — NO SCALE



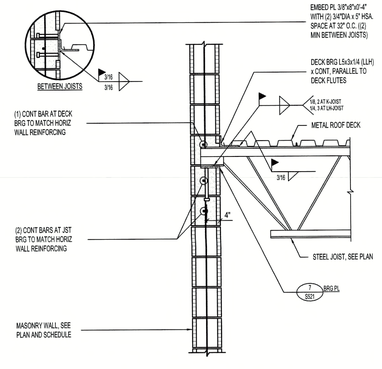
10 TYPICAL JOIST (5" BEARING ENDS)/DECK BEARING AT 8" MASONRY WALLS — NO SCALE



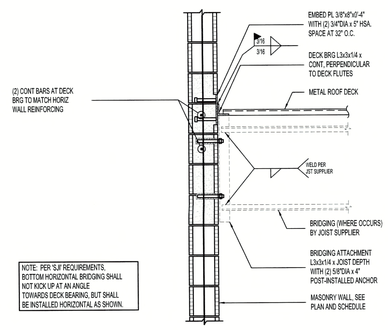
11 TYPICAL JOIST (5" BEARING END)/DECK BEARING AT 8" MASONRY WALLS — NO SCALE



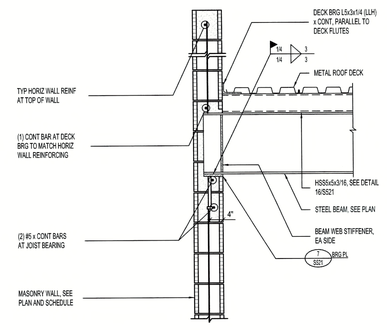
12 TYPICAL DECK BEARING AT 8" MASONRY WALLS WITH BRIDGING ATTACHMENT — NO SCALE



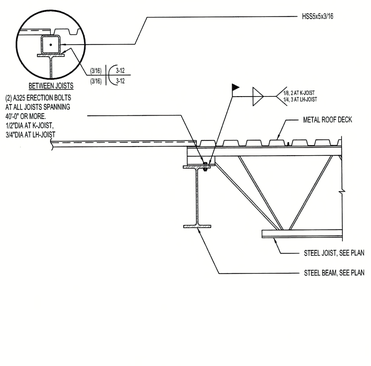
13 TYPICAL JOIST (5" BEARING ENDS) BEARING AT 8" MASONRY WALLS — NO SCALE



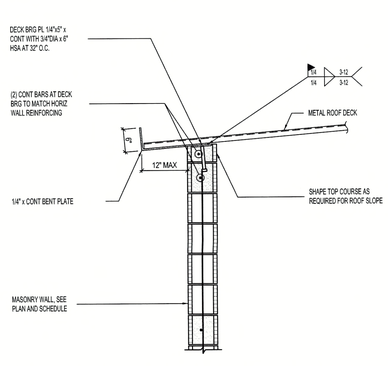
14 TYPICAL DECK BEARING WITH BRIDGING ATTACHMENT AT 8" MASONRY WALLS — NO SCALE



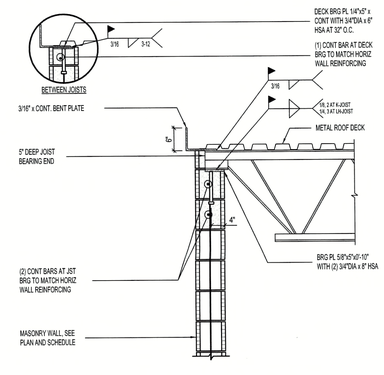
15 TYPICAL STEEL BEAM/DECK BEARING DETAIL AT 8" MASONRY WALLS — NO SCALE



16 TYPICAL JOIST (5" BEARING ENDS) BEARING AT STEEL BEAM — NO SCALE



17 TYPICAL DECK BEARING AT 8" OR 10" MASONRY WALLS — NO SCALE



18 TYPICAL JOIST (5" BEARING END)/DECK BEARING AT 8" MASONRY WALLS — NO SCALE

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12-1-2017

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project:
Barker Park Amphitheater

North Ogden, Utah

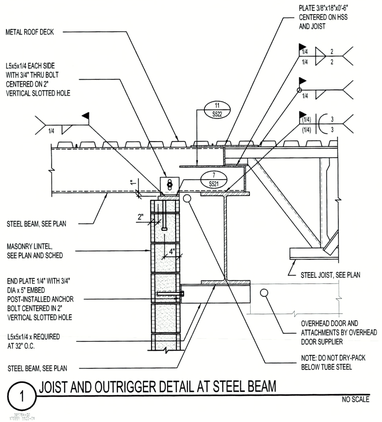
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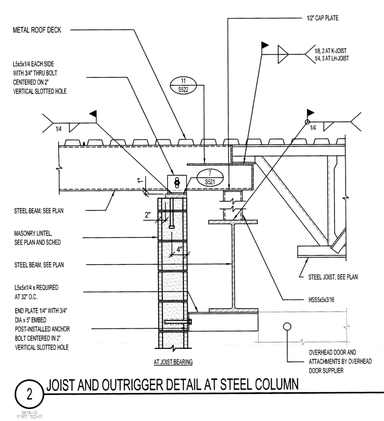
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FRAMING DETAILS

sheet:
S521

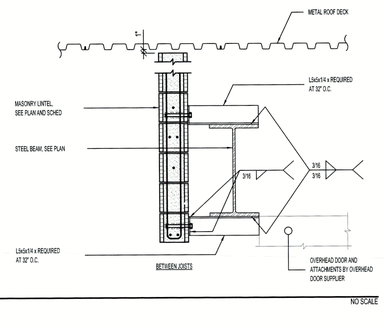
Phase 1 Bid Package 2

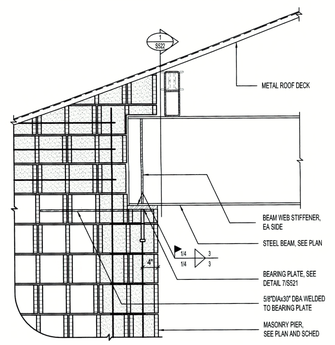


1 JOIST AND OUTRIGGER DETAIL AT STEEL BEAM — NO SCALE

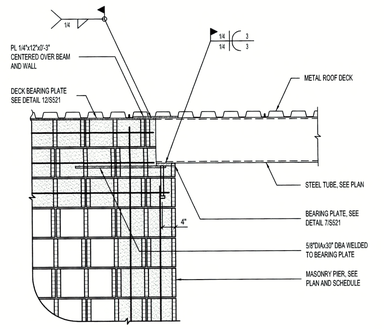


2 JOIST AND OUTRIGGER DETAIL AT STEEL COLUMN — NO SCALE

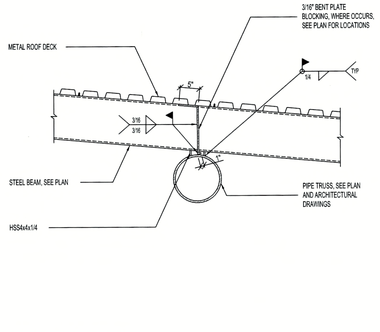




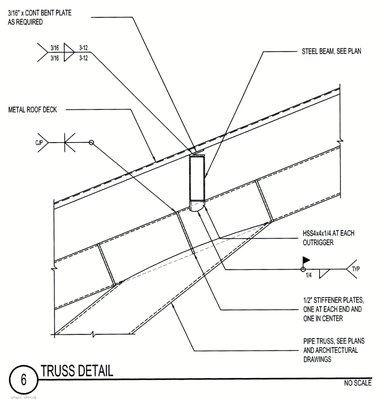
3 STEEL BEAM/JOIST BEARING AT MASONRY PIER — NO SCALE



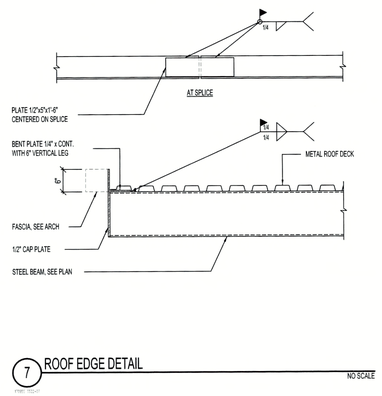
4 STEEL TUBE BEARING AT MASONRY PIER — NO SCALE



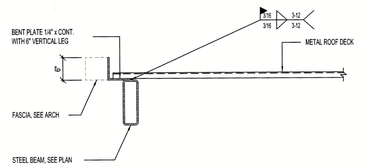
5 OUTRIGGER DETAIL — NO SCALE



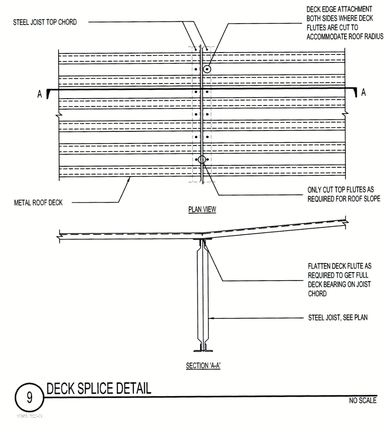
6 TRUSS DETAIL — NO SCALE



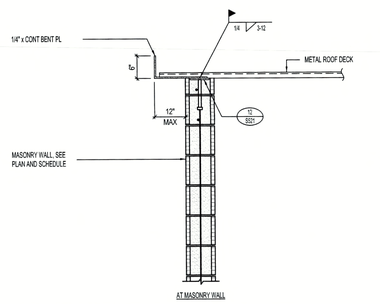
7 ROOF EDGE DETAIL — NO SCALE

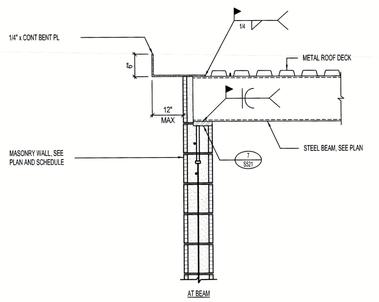


8 ROOF EDGE DETAIL — NO SCALE

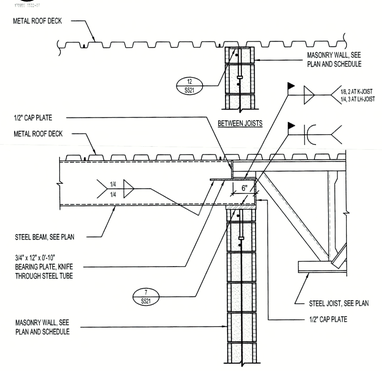


9 DECK SPLICE DETAIL — NO SCALE





10 DECK EDGE DETAIL [OVERHANG < 12"] — NO SCALE



11 JOIST AND OUTRIGGER DETAIL AT MASONRY WALL — NO SCALE

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project:

Barker Park Amphitheater

North Ogden, Utah

project#: 170955
date: 01 December 2017

revisions :

title:

FRAMING DETAILS

sheet:

S522

Phase 1 Bid Package 2

CONCRETE FOOTING SCHEDULE

MARK	WIDTH	LENGTH	DEPTH	REINFORCING CROSSWISE				REINFORCING LENGTHWISE				COMMENTS
				No.	SIZE	LENGTH	SPACING	No.	SIZE	LENGTH	SPACING	
FTS2.0	2'-0"	CONT	12"	-	#4	1'-6"	48"	3	#4	CONT	EQ	THICKENED SLAB
FC1.5	1'-6"	CONT	12"	-	#4	1'-0"	48"	2	#5	CONT	EQ	.
FC2.0	2'-0"	CONT	12"	-	#4	1'-6"	48"	3	#4	CONT	EQ	
FC2.5	2'-6"	CONT	12"	-	#5	2'-0"	14"	3	#5	CONT	EQ	
FC3.0	3'-0"	CONT	12"	-	#5	2'-6"	14"	3	#5	CONT	EQ	
FC3.5	3'-6"	CONT	12"	-	#5	3'-0"	14"	3	#5	CONT	EQ	
FC4.0	4'-0"	CONT	12"	-	#5	3'-6"	14"	4	#5	CONT	EQ	
FC4.5	4'-6"	CONT	12"	-	#5	4'-0"	14"	4	#5	CONT	EQ	
FS3.0	3'-0"	3'-0"	12"	3	#5	2'-6"	EQ	3	#5	2'-6"	EQ	
FS3.5	3'-6"	3'-6"	12"	3	#5	3'-0"	EQ	3	#5	3'-0"	EQ	
FS4.0	4'-0"	4'-0"	12"	4	#5	3'-6"	EQ	4	#5	3'-6"	EQ	
FS4.5	4'-6"	4'-6"	12"	4	#5	4'-0"	EQ	4	#5	4'-0"	EQ	
FS5.0	5'-0"	5'-0"	12"	5	#5	4'-6"	EQ	5	#5	4'-6"	EQ	
FS5.5	5'-6"	5'-6"	12"	5	#5	5'-0"	EQ	5	#5	5'-0"	EQ	
FS6.0	6'-0"	6'-0"	13"	6	#5	5'-6"	EQ	6	#5	5'-6"	EQ	
FS6.5	6'-6"	6'-6"	14"	7	#5	6'-0"	EQ	7	#5	6'-0"	EQ	
FS7.0	7'-0"	7'-0"	15"	8	#5	6'-6"	EQ	8	#5	6'-6"	EQ	
FS7.5	7'-6"	7'-6"	16"	7	#6	7'-0"	EQ	7	#6	7'-0"	EQ	
FS8.0	8'-0"	8'-0"	17"	8	#6	7'-6"	EQ	8	#6	7'-6"	EQ	
FS8.5	8'-6"	8'-6"	18"	8	#6	8'-0"	EQ	8	#6	8'-0"	EQ	
FS9.0	9'-0"	9'-0"	19"	9	#6	8'-6"	EQ	9	#6	8'-6"	EQ	
FS9.5	9'-6"	9'-6"	20"	10	#6	9'-0"	EQ	10	#6	9'-0"	EQ	
FS10.0	10'-0"	10'-0"	21"	11	#6	9'-6"	EQ	11	#6	9'-6"	EQ	
FS10.5	10'-6"	10'-6"	22"	9	#7	10'-0"	EQ	9	#7	10'-0"	EQ	
FS11.0	11'-0"	11'-0"	23"	10	#7	10'-6"	EQ	10	#7	10'-6"	EQ	
FS11.5	11'-6"	11'-6"	23"	11	#7	11'-0"	EQ	11	#7	11'-0"	EQ	
FS12.0	12'-0"	12'-0"	24"	12	#7	11'-6"	EQ	12	#7	11'-6"	EQ	
FS12.5	12'-6"	12'-6"	26"	11	#8	12'-0"	EQ	11	#8	12'-0"	EQ	
FS13.0	13'-0"	13'-0"	27"	11	#8	12'-6"	EQ	11	#8	12'-6"	EQ	
FS13.5	13'-6"	13'-6"	27"	11	#8	13'-0"	EQ	11	#8	13'-0"	EQ	
FS14.0	14'-0"	14'-0"	28"	12	#8	13'-6"	EQ	12	#8	13'-6"	EQ	
FS14.5	14'-6"	14'-6"	29"	13	#8	14'-0"	EQ	13	#8	14'-0"	EQ	
FS15.0	15'-0"	15'-0"	30"	11	#9	14'-6"	EQ	11	#9	14'-6"	EQ	
FS15.5	15'-6"	15'-6"	30"	12	#9	15'-0"	EQ	12	#9	15'-0"	EQ	
FS16.0	16'-0"	16'-0"	31"	13	#9	15'-6"	EQ	13	#9	15'-6"	EQ	
FS16.5	16'-6"	16'-6"	32"	13	#9	16'-0"	EQ	13	#9	16'-0"	EQ	
FS17.0	17'-0"	17'-0"	33"	14	#9	16'-6"	EQ	14	#9	16'-6"	EQ	
FS17.5	17'-6"	17'-6"	34"	15	#9	17'-0"	EQ	15	#9	17'-0"	EQ	

CONCRETE FOOTING NOTES:
1. PLACE ALL FOOTING REINFORCING IN THE BOTTOM OF THE FOOTING WITH 3" CLEAR CONCRETE COVER (UNO).
2. TOP REINFORCING, WHERE OCCURS, SHALL BE PLACED IN THE TOP OF THE FOOTING WITH 2" MINIMUM CONCRETE COVER.
3. IF FOOTINGS ARE EARTH-FORMED, FOOTINGS SHALL BE 6" LONGER AND WIDER THAN SCHEDULED.
4. RUN CONTINUOUS FOOTING REINFORCEMENT THROUGH SPOT FOOTINGS.
5. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.
6. SOME SCHEDULED FOOTINGS MAY NOT BE USED, SEE FOOTING AND FOUNDATION PLAN FOR FOOTING MARKS.

1 CONCRETE FOOTING SCHEDULE — NO SCALE

CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE

BAR SIZE	f'c = 3000psi & f'c = 3500psi				f'c = 4000psi & f'c = 4500 psi				f'c = 5000psi				f'c = 6000psi			
	REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		REGULAR		TOP	
	CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS		CLASS	
	A	B	A	B	A	B	A	B	A	B	A	B	A	B	A	B
#3	13"	17"	17"	21"	12"	16"	16"	21"	12"	16"	16"	21"	12"	16"	16"	21"
#4	17"	22"	22"	28"	15"	19"	19"	25"	13"	17"	17"	22"	12"	16"	16"	21"
#5	21"	27"	27"	35"	18"	24"	24"	31"	16"	21"	21"	27"	15"	19"	19"	25"
#6	27"	36"	36"	46"	24"	31"	31"	40"	21"	28"	28"	36"	20"	25"	25"	33"
#7	37"	48"	48"	63"	32"	42"	42"	54"	29"	38"	38"	49"	27"	34"	34"	44"
#8	49"	64"	64"	82"	42"	55"	55"	71"	38"	49"	49"	64"	35"	45"	45"	58"
#9	62"	80"	80"	104"	54"	70"	70"	90"	48"	62"	62"	81"	44"	57"	57"	74"
#10	78"	102"	102"	132"	68"	88"	88"	115"	61"	79"	79"	102"	56"	72"	72"	94"
#11	96"	125"	125"	162"	83"	108"	108"	141"	76"	97"	97"	126"	68"	88"	88"	115"

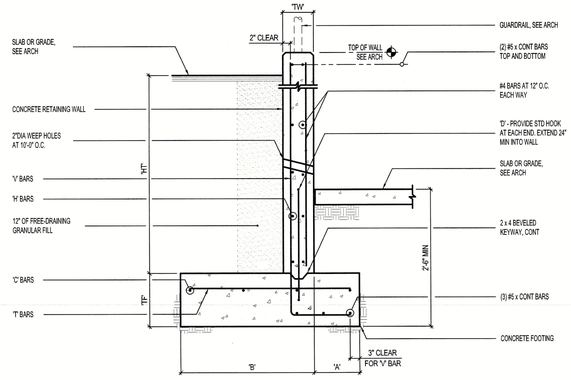
CONCRETE REINFORCING BAR LAP SPLICE NOTES:
1. THIS SCHEDULE SHALL BE USED FOR ALL BAR SPLICES IN CONCRETE WALLS, UNLESS NOTED OTHERWISE.
2. CLASS 'A' SPLICES MAY BE USED ONLY IN CASES WHERE 50% OR LESS OF THE BARS ARE SPLICED WITHIN THE LAP SPLICE LENGTH.
3. CLASS 'B' SPLICES SHALL BE USED FOR ALL SPLICES UNLESS THE REQUIREMENTS OF NOTE No. 2 ABOVE ARE MET.
4. TIES AND STIRRUPS SHALL NOT BE SPLICED.
5. DO NOT SPLICE VERTICAL BARS IN RETAINING WALLS UNLESS SPECIFICALLY SHOWN.
6. SPLICES FOR BUNDLED BARS:
   a. FOR BUNDLED BARS OF THREE OR LESS, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.2.
   b. FOR BUNDLED BARS OF FOUR OR MORE, LAP SPLICE LENGTHS SHALL BE MULTIPLIED BY 1.33.
   c. INDIVIDUAL BAR SPLICES WITHIN A BUNDLE SHALL NOT OVERLAP.
   d. ENTIRE BUNDLES SHALL NOT BE LAP SPLICED.
7. FOR ALL LIGHTWEIGHT CONCRETE, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3.
8. FOR ALL EPOXY COATED BARS, LAP LENGTHS SHALL BE MULTIPLIED BY 1.3 FOR TOP BARS AND 1.5 FOR REGULAR BARS.
9. TOP BARS ARE CLASSIFIED AS HORIZONTAL BARS WHERE 12", OR MORE, OF FRESH CONCRETE IS CAST BELOW THE REINFORCING BAR.
10. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

3 CONCRETE REINFORCING BAR LAP SPLICE SCHEDULE — NO SCALE

CONCRETE RETAINING WALL SCHEDULE

MARK	HT	A	B	TW	TF	V		H		T		C		D		COMMENTS
						SIZE	SPACE	SIZE	SPACE	SIZE	SPACE	SIZE	SPACE	SIZE	SPACE	
CRW-12	12'-0"	3'-0"	7'-0"	12"	20"	#8	14"	#4	12"	#6	12"	#6	12"	-	-	-
CRW-10	10'-0"	2'-0"	5'-6"	10"	15"	#6	10"	#4	12"	#6	13"	#6	13"	-	-	
CRW-8	8'-0"	1'-6"	4'-0"	8"	14"	#5	12"	#4	12"	#5	19"	#5	10"	-	-	
CRW-6	6'-0"	1'-0"	2'-6"	8"	14"	#5	14"	#4	12"	#5	12"	#5	12"	-	-	

CONCRETE RETAINING WALL NOTES:
1. 'V' BARS SHALL NOT BE SPLICED BELOW MID-HEIGHT OF WALL.
2. WHERE 'HT' IS GREATER THAN 9'-0", THEN ONE HALF OF THE 'V' BARS MAY BE DISCONTINUED AT MID-HEIGHT OF WALL.
3. PROVIDE VERTICAL CONTRACTION JOINTS AT 20'-0" O.C. MAXIMUM. SEE ARCHITECTURAL DRAWINGS AND GENERAL STRUCTURAL NOTES.
4. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



4 CONCRETE RETAINING WALL SCHEDULE — NO SCALE

CONCRETE WALL SCHEDULE

MARK	THICKNESS	REINFORCING			WALL TYPE	COMMENTS
		VERTICAL	HORIZONTAL	TOP AND BOTTOM		
CW-8A	8"	#4 AT 16" O.C.	#4 AT 12" O.C.	(1) #4	A	ALTERNATE VERTICAL BARS WITH MASONRY WALL REINFORCING
CW-8B	8"	#5 AT 12" O.C., E.F.	#4 @ 12" O.C.	(1) #5	B	
CW-16A	16"	#4 @ 18" O.C., E.F.	#4 @ 12" O.C., E.F.	(2) #4	C	
CW-24A	24"	(3) #4 @ 18" O.C.	(3) #4 @ 12" O.C.	(3) #4	D	

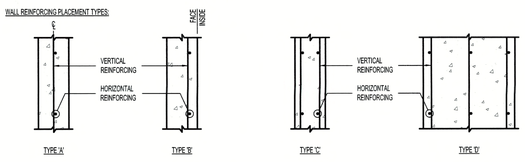
ABBREVIATIONS:
E.F. EACH FACE
I.F. INSIDE FACE
O.F. OUTSIDE FACE

WALLS NOT DESIGNATED IN PLAN

THICKNESS	REINFORCING	
	VERTICAL	HORIZONTAL
6"	#4 AT 16" O.C.	#4 AT 16" O.C.
8"	#4 AT 16" O.C.	#4 AT 12" O.C.
10"	#4 AT 16" O.C.	#5 AT 15" O.C.
12"	#4 AT 16" O.C. E.F.	#4 AT 16" O.C. E.F.

CONCRETE FOUNDATION WALL NOTES:
1. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

WALL REINFORCING PLACEMENT TYPES:

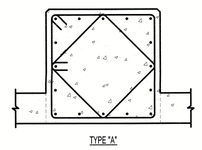


2 CONCRETE WALL SCHEDULE — NO SCALE

CONCRETE PIER SCHEDULE

MARK	PIER SIZE	REINFORCING		TYPE	COMMENTS
		VERTICAL	TIES		
CP-30A	30"x30"	(16) #8	#3 @ 8" O.C.	A	.

CONCRETE PIER NOTES:
1. INSTALL (3) SETS OF TIES WITHIN TOP 5" OF ALL PIERS (UNO).
2. RUN HORIZONTAL CONCRETE WALL REINFORCING CONTINUOUS THROUGH PIER WHEN PIER IS POURED MONOLITHICALLY WITH CONCRETE WALL.
3. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



5 CONCRETE PIER SCHEDULE — NO SCALE

LICENSED PROFESSIONAL STRUCTURAL ENGINEER
BRETT KARL GOODMAN
No. 7991094-2203
STATE OF UTAH
12-1-2017

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CONCRETE SCHEDULES

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Phase 1 Bid Package 2

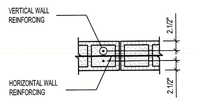
MASONRY WALL SCHEDULE

MARK	THICKNESS	MATERIAL	SOLID GROUT	REINFORCING			COMMENTS
				VERTICAL	HORIZONTAL	JOINTS	
MW-8A	8"	CMU	YES	#5 AT 32" O.C.	#4 AT 24" O.C.	NONE	-
MW-8B	8"	CMU	YES	#5 AT 16" O.C.	#4 AT 24" O.C.	NONE	
MW-8C	8"	CMU	NO	#5 AT 32" O.C.	#5 AT 48" O.C.	NONE	

MASONRY WALLS NOT DESIGNATED IN PLAN

THICKNESS	REINFORCING		
	VERTICAL	HORIZONTAL (NOT SOLID GROUTED)	HORIZONTAL (SOLID GROUTED)
6"	#5 AT 32" O.C.	#4 AT 48" O.C.	#4 AT 24" O.C.
8"	#5 AT 32" O.C.	#5 AT 48" O.C.	#4 AT 24" O.C.
10"	#5 AT 24" O.C.	#6 AT 48" O.C.	#5 AT 24" O.C.
12"	#5 AT 24" O.C.	(2) #5 AT 48" O.C.	(2) #4 AT 24" O.C.

MASONRY WALL NOTES:
1. COORDINATE WALL FINISHES, MATERIALS, COURSING, ETC. WITH ARCHITECTURAL DRAWINGS.
2. DO NOT SOLID GROUT WALLS UNLESS REQUIRED BY SCHEDULE, NOTES, OR DETAILS.
3. SOLID GROUT ALL MASONRY COURSES BELOW GRADE.
4. VERTICAL REINFORCING SHALL BE CENTERED IN WALL (UNO).
5. PROVIDE TWO VERTICAL BARS (MIN) AT ALL CORNERS AND END OF WALLS.
6. HORIZONTAL WALL REINFORCING SHALL BE PLACED BETWEEN VERTICAL MASONRY COLUMN REINFORCING BARS.
7. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
8. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



DOUBLE VERTICAL LAYER WALL REINFORCING

1 MASONRY WALL SCHEDULE — NO SCALE

MASONRY REINFORCING LAP SCHEDULE

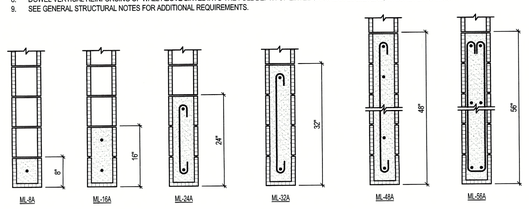
BAR SIZE	(1) BAR PER CELL	(2) BARS PER CELL
#3	13"	13"
#4	21"	21"
#5	34"	34"
#6	37"	USE MECH SPLICE COUPLER
#7	USE MECH SPLICE COUPLER	USE MECH SPLICE COUPLER
#8	USE MECH SPLICE COUPLER	USE MECH SPLICE COUPLER

2 MASONRY REINFORCING LAP SCHEDULE (2000psi) — NO SCALE

MASONRY LINTEL SCHEDULE

MARK	DEPTH	MAXIMUM SPAN FOR UNSCHEDULED OPENINGS	REINFORCING		COMMENTS
			HORIZONTAL	STIRRUPS	
ML-8A	8"	3'-4"	(1) #5 x CONT	NONE	-
ML-16A	16"	6'-0"	(1) #5 x CONT TOP AND BOTTOM	NONE	
ML-24A	24"	8'-0"	(1) #5 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	
ML-32A	32"	10'-0"	(1) #7 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	
ML-48A	48"	-	(1) #7 x CONT TOP AND BOTTOM	#4 AT 8" O.C.	
ML-56A	56"	-	(2) #7x CONT TOP AND BOTTOM AND CENTER	#4 AT 8" EACH END FOR 6'-0" #4 AT 24" O.C. ELSEWHERE	STAGE LINTEL, SEE SCHEMATIC REINFORCE AS MW-8C

MASONRY LINTEL NOTES:
1. LINTEL WIDTH AND MATERIAL TYPE SHALL BE THE SAME AS THE WALL IN WHICH THE LINTEL IS CONSTRUCTED.
2. GROUT MASONRY LINTELS MONOLITHICALLY WITH THE SUPPORT WALL OR COLUMN AT EACH END.
3. MASONRY LINTELS ML-8A, ML-16A, ML-24A, AND ML-32A SHALL BE USED OVER OPENINGS IN MASONRY WALLS WHEN A SPECIFIC MASONRY LINTEL IS NOT OTHERWISE SPECIFIED. WHEN A LINTEL IS SPECIFIED ON THE PLANS, THE MAXIMUM SPAN AS NOTED IN THIS SCHEDULE SHALL NOT APPLY. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SPECIFIED ON THE PLANS WHICH HAVE A SPAN GREATER THAN 10'-0".
4. MASONRY LINTELS ML-8A, ML-16A, ML-24A, AND ML-32A SHALL NOT BE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDERS UNLESS NOTED OTHERWISE ON THE PLANS. JOISTS SHALL NOT BEAR ON ANY LINTEL LESS THAN 16" DEEP. CONSULT THE STRUCTURAL ENGINEER FOR LINTELS NOT SHOWN ON THE PLANS WHICH ARE LOCATED DIRECTLY BELOW FLOOR OR ROOF BEAMS OR GIRDERS.
5. EXTEND ALL HORIZONTAL REINFORCING 48 BAR DIAMETERS MINIMUM BEYOND THE EDGE OF ALL OPENINGS. IF HORIZONTAL REINFORCING CANNOT EXTEND 48 BAR DIAMETERS BEYOND EDGE OF OPENING, PROVIDE 90° STANDARD HOOK.
6. SPLICE TOP BARS AT MIDSPAN OF LINTEL ONLY AND BOTTOM BARS OVER SUPPORTS ONLY.
7. HORIZONTAL WALL REINFORCING SHALL CONTINUE THROUGH MASONRY LINTELS. WHERE BOTH HORIZONTAL WALL REINFORCING AND LINTEL REINFORCING OCCUR IN THE SAME COURSE, USE THE LARGER REINFORCING.
8. DOWEL VERTICAL REINFORCING OF WALL ABOVE LINTEL INTO THE FULL DEPTH OF LINTEL OR 48 BAR DIAMETERS, WHICHEVER IS LESS.
9. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.

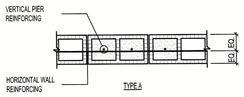


4 MASONRY LINTEL SCHEDULE — NO SCALE

MASONRY PIER SCHEDULE

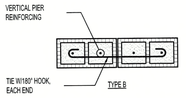
MARK	SIZE	REINFORCING		REINFORCING SCHEMATIC	COMMENTS
		VERTICAL	TIES		
MP-16A	WT x 16"	(2) #5	NONE		
MP-16B	WT x 16"	(2) #5	#3 AT 8" O.C.		SEE NOTE No. 6
MP-24A	WT x 24"	(3) #5	NONE		
MP-32A	WT x 32"	(4) #5	NONE		
MP-32B	WT x 32"	(4) #5	#3 AT 8" O.C.		SEE NOTE No. 6
MP-40B	WT x 40"	(5) #5	#3 AT 8" O.C.		SEE NOTE No. 6
MP-48A	WT x 48"	(6) #5	NONE		
MP-48B	WT x 48"	(6) #5	#3 AT 16" O.C.		SEE NOTE No. 6
MP-56A	WT x 56"	(7) #5	NONE		
MP-56B	WT x 56"	(7) #5	#3 AT 16" O.C.		SEE NOTE No. 6
MP-72B	WT x 72"	(9) #5	#4 AT 24" O.C.		SEE NOTE No. 6
MP24x64A	24" x 64"	(16) #5	#3 AT 8" O.C.	SEE SCHEMATIC	

MASONRY PIER NOTES:
1. HORIZONTAL WALL REINFORCING SHALL BE LOCATED TO THE INSIDE OF THE VERTICAL BARS FOR DOUBLE LAYER MASONRY PIERS.
2. VERTICAL REINFORCING AND TIES SHALL EXTEND FULL HEIGHT OF WALL (UNO).
3. VERTICAL MASONRY PIER REINFORCING SHALL EXTEND INTO THE FOOTING AND TERMINATE WITH A STANDARD 90° HOOK. FOR CONCRETE FOUNDATION WALLS TALLER THAN 5'-0", VERTICAL PIER REINFORCING SHALL DOWEL 4'-0" MINIMUM INTO THE FOUNDATION WALL (UNO).
4. IN CONCRETE FOUNDATION WALLS, VERTICAL MASONRY PIER REINFORCING SHALL BE TIED WITH #3 TIES AT THE SAME SPACING AND CONFIGURATION AS THE MASONRY PIER ABOVE.
5. HORIZONTAL REINFORCING OF ADJACENT WALLS SHALL RUN CONTINUOUS THROUGH MASONRY PIERS.
6. WHERE NOTED IN SCHEDULE TIES EXTEND FROM BOTTOM TO TOP OF OPENING AND REPLACE HORIZONTAL WALL REINFORCING.
7. SEE GENERAL STRUCTURAL NOTES FOR ADDITIONAL REQUIREMENTS.



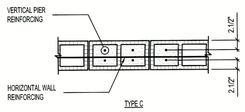
TYPE A

TYPICAL SINGLE LAYER PIER CONFIGURATION SCHEMATIC



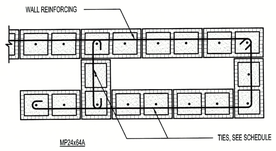
TYPE B

TYPICAL ISOLATED SINGLE LAYER PIER CONFIGURATION SCHEMATIC



TYPE C

TYPICAL DOUBLE LAYER PIER CONFIGURATION SCHEMATIC



MP24x64A

3 MASONRY PIER SCHEDULE — NO SCALE

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No. 7991094-2203
LICENSED PROFESSIONAL STRUCTURAL ENGINEER
STATE OF UTAH

12-1-2017

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project:
Barker Park Amphitheater

North Ogden, Utah

project#: 170955
date: 01 December 2017

revisions :

title:
MASONRY SCHEDULES

sheet:
S602

Phase 1 Bid Package 2

MISC. SYMBOL LEGEND

SYMBOL	DESCRIPTION
# SHEET	DETAIL INDICATOR: # INDICATES DETAIL NUMBER, SHEET INDICATES DRAWING SHEET WHERE DETAIL IS SHOWN.
# SHEET	ELEVATION OR SECTION INDICATOR, EXTERIOR: # INDICATES ELEVATION OR SECTION NUMBER, SHEET INDICATES DRAWING SHEET WHERE ELEVATION OR SECTION IS SHOWN.
100	ROOM OR SPACE NUMBER.
1	KEYNOTE INDICATOR.
	REVISION INDICATOR.
CU-1	EQUIPMENT INDICATOR.
P-	PLUMBING FIXTURE INDICATOR.
TYPE CFM SIZE	DIFFUSER/GRILLE INDICATOR.
TYPE SIZE	DIFFUSER/GRILLE INDICATOR.
	BREAK, STRAIGHT
	BREAK, ROUND.
	MATCH LINE INDICATOR
	HIDDEN FEATURES LINE: HIDDEN, THIN LINE.
	CONTRACT LIMIT LINE: DASHDOT, WIDE LINE.
	NEW CONNECTION POINT TO EXISTING

PLUMBING SYMBOL LEGEND

SYMBOL	DESCRIPTION
C.B.	CATCH BASIN
M.H.	MANHOLE
W.H.	WALL HYDRANT
H.B.	HOSE BIBB
	CLEANOUT TO GRADE
	FLOOR CLEANOUT
	WALL CLEANOUT
	1/2 GRATE
	3/4 GRATE
	FULL GRATE

DEFINITIONS

NOTE: ALL DEFINITIONS MAY NOT BE USED.

INDICATED: THE TERM "INDICATED" REFERS TO GRAPHIC REPRESENTATIONS, NOTES, OR SCHEDULES ON THE DRAWINGS, OTHER PARAGRAPHS OR SCHEDULES IN THE SPECIFICATIONS, AND SIMILAR REQUIREMENTS IN THE CONTRACT DOCUMENTS. WHERE TERMS SUCH AS "SHOWN", "NOTED", "SCHEDULED", AND "SPECIFIED" ARE USED, IT IS TO HELP THE READER LOCATE THE REFERENCE, NO LIMITATION ON LOCATION IS INTENDED.

DIRECTED: TERMS SUCH AS "DIRECTED", "REQUESTED", AUTHORIZED", "SELECTED", "APPROVED", "REQUIRED", AND "PERMITTED" MEAN "DIRECTED BY THE ENGINEER", "REQUESTED BY THE ENGINEER", AND SIMILAR PHRASES.

APPROVED: THE TERM "APPROVED", WHERE USED IN CONJUNCTION WITH THE ENGINEER'S ACTION ON THE CONTRACTOR'S SUBMITTALS, APPLICATIONS, AND REQUESTS, IS LIMITED TO THE ENGINEER'S DUTIES AND RESPONSIBILITIES AS STATED IN GENERAL AND SUPPLEMENTARY CONDITIONS.

FURNISH: THE TERM "FURNISH" IS USED TO MEAN "SUPPLY AND DELIVER TO THE PROJECT SITE, READY FOR UNLOADING, UNPACKING, ASSEMBLY, INSTALLATION, AND SIMILAR OPERATIONS."

INSTALL: THE TERM "INSTALL" IS USED TO DESCRIBE OPERATIONS AT PROJECT SITE INCLUDING THE ACTUAL "UNLOADING, UNPACKING, ASSEMBLY, ERECTION, PLACING, ANCHORING, APPLYING, WORKING TO DIMENSION, FINISHING, CURING, PROTECTING, CLEANING, AND SIMILAR OPERATIONS."

PROVIDE: THE TERM "PROVIDE" MEANS "TO FURNISH AND INSTALL, COMPLETE AND READY FOR THE INTENDED USE."

INSTALLER: AN "INSTALLER" IS THE CONTRACTOR OR AN ENTITY ENGAGED BY THE CONTRACTOR, EITHER AS AN EMPLOYEE, SUBCONTRACTOR, OR SUB-SUBCONTRACTOR, FOR PERFORMANCE OF A PARTICULAR CONSTRUCTION ACTIVITY, INCLUDING INSTALLATION, ERECTION, APPLICATION, AND SIMILAR OPERATIONS. INSTALLERS ARE REQUIRED TO BE EXPERIENCED IN THE OPERATIONS THEY ARE ENGAGED TO PERFORM.

PLUMBING PIPING LEGEND

SYMBOL	DESCRIPTION
CWV	COMBINATION WASTE AND VENT
	SOIL, WASTE - ABOVE GRADE (SW)
	SOIL, WASTE - BELOW GRADE (SW)
GW	GREASE WASTE - ABOVE GRADE
GW	GREASE WASTE - BELOW GRADE
	VENT (V)
AV	ACID VENT
AW	ACID WASTE - ABOVE GRADE
AW	ACID WASTE - BELOW GRADE
	DOMESTIC COLD WATER (DCW)
	DOMESTIC HOT WATER (DHW)
	DOMESTIC HOT WATER RECIRC (DHWR)
180	180°F HOT WATER
180R	180° HOT WATER RETURN
160	160° HOT WATER
160R	160° HOT WATER RETURN
RW	RAINWATER - ABOVE GRADE
RW	RAINWATER - BELOW GRADE
SRW	SECONDARY RAINWATER ABOVE GRADE
SRW	SECONDARY RAINWATER BELOW GRADE
SD	STORM DRAIN
VTR	VENT THRU ROOF
/	NON POTABLE WATER
(E)	EXISTING PIPE
(E)	EXISTING PIPE TO BE REMOVED
IW	IRRIGATION WATER
SS	SANITARY SEWER
W	WATER
PWS	PURE WATER SUPPLY
PWR	PURE WATER RETURN
G	GAS
FP	FIRE PROTECTION
LPG	PROPANE
VAC	VACUUM
A	COMPRESSED AIR
MA	MEDICAL AIR
O	OXYGEN
NO	NITROUS OXIDE
N	NITROGEN
CO2	CARBON DIOXIDE
EVAC	EVACUATION

SYMBOL LEGEND

SYMBOL	DESCRIPTION
	VALVES, METERS, AND GAUGES
	SHUT OFF VALVE
	GATE VALVE
	CHECK VALVE
	AUTO 2-WAY VALVE
	AUTO 3-WAY VALVE
	GLOBE VALVE
	BALL VALVE
	RELIEF VALVE
	CHAIN OPERATED GATE VALVE
	PRESSURE REDUCING VALVE
	BUTTERFLY VALVE
	SOLENOID VALVE
	ANGLE VALVE
	VENTURI
	BALANCING OR PLUG COCK
	FLOW SETTER
	EXPANSION VALVE (REFRIG.)
	GAS COCK
MAV	MANUAL AIR VENT
	STRAINER
	GAUGE COCK
	FLEXIBLE CONNECTION
	PRESSURE GAUGE
	THERMOMETER
	VICTUALIC COUPLING
	REDUCER CONCENTRIC
	REDUCER ECCENTRIC
	REFRIGERANT SITE GLASS
	REFRIGERANT STRAINER
	REFRIGERANT FILTER DRIER
	90 DEG ELBOW UP
	90 DEG ELBOW DOWN
	90 DEG TEE UP
	90 DEG TEE DOWN
	UNION
	CAPPED PIPE
	ANCHOR
	FLOAT AND THERMOSTATIC TRAP

ABBREVIATIONS

NOTE: ALL ABBREVIATIONS MAY NOT BE USED.

(E)	EXISTING
(F)	FUTURE
AD	ACCESS DOOR
AIR COND	AIR CONDITION(-ING,-ED)
APD	AIR PRESSURE DROP
BD	BALANCING DAMPER
BHP	BRAKE HORSE POWER
BTU	BRITISH THERMAL UNIT
BTUH	BTU/HOUR
CFH	CUBIC FEET PER HOUR
CFM	CUBIC FEET PER MINUTE
CLG	COOLING
COMP	COMPONENT
COND	CONDENS(-ER, -ING, -ATION)
CV	CONTROL VALVE
DB	DRY BULB TEMPERATURE
DCW	DOMESTIC COLD WATER
DHW	DOMESTIC HOT WATER
DHWR	DOMESTIC HOT WATER RECIRC
DIA	DIAMETER
DISCH	DISCHARGE
DP	DEPTH OR DEEP
EA	EXHAUST AIR
EER	ENERGY EFFICIENCY RATIO
EFF	EFFICIENCY
EG	ETHYLENE GLYCOL
ELEC	ELECTRIC
ELEV	ELEVATION
ENT	ENTERING
EVAP	EVAPORAT(-E, -ING, -ED, -OR)
EWT	ENTERING WATER TEMPERATURE
EXT	EXTERNAL
FC	FLEXIBLE CONNECT(-OR, -ION)
FD	FIRE DAMPER
FPM	FEET PER MINUTE
FPS	FEET PER SECOND
FSD	FIRE SMOKE DAMPER
GAL	GALLON(S)
GE	GREASE EXHAUST
GPH	GALLONS PER HOUR
GPM	GALLONS PER MINUTE
HD	HEAD
HG	MERCURY
HP	HORSEPOWER
HR	HOUR
HT	HEIGHT
HTG	HEATING
HZ	HERTZ (FREQUENCY)
ID	INSIDE DIAMETER
IN	INCH
KW	KILOWATT
LAT	LEAVING AIR TEMPERATURE
LBS	POUNDS
LG	LENGTH
LRA	LOCKED ROTOR AMPS
LVG	LEAVING
LWT	LEAVING WATER TEMPERATURE
MBH	THOUSAND BTU PER HOUR
MCA	MINIMUM CIRCUIT AMPS
MFR	MANUFACTUR(-ER, -ED)
NC	NORMALLY CLOSED
NC	NOISE CRITERIA
NIC	NOT IN CONTRACT
NO	NORMALLY OPEN
NPSH	NET POSITIVE SUCTION HEAD
NTS	NOT TO SCALE
OA	OUTSIDE AIR
OD	OUTSIDE DIAMETER
OZ	OUNCE
PD	PRESSURE DROP OR DIFFERENCE
PG	PROPOLENE GLYCOL
PH	PHASE
PPM	PARTS PER MILLION
PRESS	PRESSURE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PSIA	PSI ABSOLUTE
PSIG	PSI GAUGE
R	THERMAL RESISTANCE
RA	RETURN AIR
RECIRC	RECIRCULATE
REFR	REFRIGERATION
REQD	REQUIRED
RLA	RATED LOAD AMPS
RPM	REVOLUTIONS PER MINUTE
SA	SUPPLY AIR
SC	SHADING COEFFICIENT
SCFM	STANDARD CUBIC FEET PER MINUTE
SCW	SOFT COLD WATER
SF	SAFETY FACTOR
SH	SENSIBLE HEAT
SL	SEA LEVEL
SP	STATIC PRESSURE
SPEC(S)	SPECIFICATION(S)
SQ	SQUARE
STD	STANDARD
SW	SOIL, WASTE
TA(R)	TRANSFER AIR (RETURN)
TA(S)	TRANSFER AIR (SUPPLY)
TD	TEMP. DROP OR DIFF.
TEMP	TEMPERATURE
THERM	THERMAL
TOT	TOTAL
TSTAT	THERMOSTAT
V	VOLT
V	VENT
VAC	VACUUM
VAV	VARIABLE AIR VOLUME
VEL	VELOCITY TEMPERATURE
VEL	VELOCITY
VENT	VENT, VENTILATION
VERT	VERTICAL
VFD	VARIABLE FREQUENCY DRIVE
VOL	VOLUME
WB	WET BULB TEMP
WC	WATER COLUMN
WG	WATER GAUGE
WPD	WATER PRESSURE DROP
WT	WEIGHT
WTR	WATER

PLUMBING GENERAL NOTES

1 THE PLUMBING DRAWINGS SHOW THE GENERAL DESIGN, ARRANGEMENT AND EXTENT OF THE PLUMBING SYSTEM. BECAUSE OF THE SMALL SCALE OF THE DRAWINGS, THESE DRAWINGS DO NOT SHOW ALL OFFSETS, BENDS OR ELBOWS NECESSARY FOR THE COMPLETE INSTALLATION IN THE SPACE PROVIDED. CONTRACTOR SHALL MAKE SUCH SLIGHT ALTERATIONS AS MAY BE NECESSARY TO MAKE THE SYSTEM COMPLETE AND OPERATIONAL IN ACCORDANCE WITH THE DESIGN INTENT. MAJOR DEVIATIONS SUCH AS CHANGES IN COMPONENT SIZES, WEIGHTS, QUANTITIES OR MATERIAL REQUIRE PRIOR APPROVAL BY THE DESIGN ENGINEER.

2 THE DRAWINGS AND SPECIFICATIONS HAVE BEEN PREPARED TO SUPPLEMENT EACH OTHER AND SHALL BE INTERPRETED AS AN INTEGRAL UNIT WITH THE ITEMS SHOWN ON ONE AND NOT THE OTHER BEING FURNISHED AND INSTALLED AS THOUGH SHOWN AND CALLED OUT IN BOTH.

3 THE ENTIRE PLUMBING INSTALLATION SHALL CONFORM TO THE REQUIREMENTS OF THE MOST RECENTLY ADOPTED BUILDING CODES, MECHANICAL CODE, PLUMBING CODE, ELECTRICAL CODE, AND ALL OTHER APPLICABLE CITY, COUNTY, STATE, AND FEDERAL CODES AND REGULATIONS IN EFFECT.

4 THE ENTIRE PLUMBING INSTALLATION SHALL CONFORM TO ANY CODES, RULES, REGULATIONS AND REQUIREMENTS OF THE BUILDING OWNER.

5 PRIOR TO FABRICATION AND INSTALLATION OF ANY PLUMBING COMPONENT THE CONTRACTOR SHALL COORDINATE THE INSTALLATION OF ALL PLUMBING WORK WITH ALL OTHER BUILDING TRADES, INCLUDING BUILDING TRADES HIRED DIRECTLY BY THE OWNER. WHERE CONFLICTS MAY OCCUR, THEY SHALL BE RESOLVED PRIOR TO INSTALLATION.

6 ALL PLUMBING INFORMATION IS NOT SHOWN ON THE PLUMBING DRAWINGS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING ALL INFORMATION ON ALL OTHER CONSTRUCTION DOCUMENTS.

7 . THE CONTRACTOR SHALL BE RESPONSIBLE TO REVIEW AND USE, WHERE APPROPRIATE, ALL THE PLUMBING DETAILS SHOWN ON THE DRAWINGS. DETAILS MAY OR MAY NOT BE CALLED OUT ON THE DRAWINGS WITH SYMBOLS OR KEYED NOTES. ANY CHANGES RESULTING FROM FAILURE TO INSTALL THE PLUMBING SYSTEM WITHOUT USING THE INCLUDED DETAILS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

8 ANY PART OF THE PLUMBING INSTALLATION THAT FAILS, IS UNFIT, OR BECOMES DAMAGED DURING CONSTRUCTION SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.

9 PROVIDE PROPER PROVISIONS FOR EXPANSION, CONTRACTION, OR MOVEMENT OF ALL PIPING.

10 PROVIDE LARGE ENOUGH PIPE SLEEVES THROUGH WALL OR FLOOR TO ALLOW FOR ANTICIPATED DIFFERENTIAL MOVEMENT.

11 ALL PIPING SHALL BE SUPPORT WITH CLEVIS HANGERS (MSS TYPE 1). PERFORATED METAL STRAPS OR PLASTIC STRAPPING (PLUMBER TAPE) SHALL NOT BE USED TO SUPPORT OR BRACE ANY PIPE.

12 PROVIDE PIPE HANGERS WITHIN 18-INCHES OF ALL CHANGES OF DIRECTION.

13 PROVIDE SWAY BRACING FOR ALL PIPING 4" AND LARGER AT ALL CHANGES IN DIRECTION GREATER THAN 45-DEGREES.

14 ALL STEEL CLEVIS HANGERS USED TO SUPPORT COPPER PIPING SHALL BE COPPER OR PLASTIC COATED.

15 COPPER PIPING SHALL NOT COME IN CONTACT WITH FIRE TREATED LUMBER. PROVIDE ½" THICK SLIP-ON CLOSED CELL INSULATION WHERE COPPER PIPING IS ADJACENT TO FIRE TREATED LUMBER. CLOSED CELL INSULATION SHALL EXTEND A MINIMUM OF 1-1/2" PAST LUMBER.

16 ALL EXPOSED PIPING SHALL BE INSTALLED IN A NEATLY ARRANGED MANNER PARALLEL TO THE BUILDING STRUCTURE.

17 ALL EXPOSED DOMESTIC WATER PIPE IN OCCUPIED SPACES SHALL BE POLISHED CHROME PLATED.

18 ALL EXPOSED DRAINAGE PIPING IN OCCUPIED SPACES INCLUDING TRAPS UNDER SINKS SHALL BE POLISHED CHROME PLATED.

19 DRAWINGS SHOW GENERAL ARRANGEMENT OF THE DRAIN WASTE AND VENT SYSTEM WITH THE REQUIRED CLEANOUTS. CONTRACTOR SHALL PROVIDE ALL ADDITIONAL CLEANOUTS AS REQUIRED BY THE PLUMBING CODE.

20 ALL SANITARY DRAINAGE SYSTEM PIPING 3" AND LARGER SHALL BE SLOPED IN DIRECTION OF FLOW AT A MINIMUM OF 1/8" PER FOOT.

21 ALL SANITARY DRAINAGE SYSTEM PIPING SMALLER THAN 3" SHALL BE SLOPED IN DIRECTION OF FLOW AT A MINIMUM OF 1/4" PER FOOT.

22 SLOPE VENT SYSTEM TOWARDS DRAINAGE SYSTEM.

23 SIMILAR EQUIPMENT SHALL BE OF THE SAME MANUFACTURER.

24 ALL EQUIPMENT SHALL PROVIDE THE SCHEDULED PERFORMANCE AT THE JOB SITE ELEVATION.

25 FIXTURE AND EQUIPMENT MODEL NUMBERS SHOWN IN PLUMBING FIXTURE SCHEDULE AND PLUMBING EQUIPMENT SCHEDULE ARE SHOWN TO ESTABLISH THE TYPE OF PRODUCT THAT SHALL BE USED. THE SELECTED PRODUCT SHALL MEET THE SCHEDULED PERFORMANCE DATA SHOWN ON THE SCHEDULE EVEN IF A DIFFERENT MODEL IS SUPPLIED THAT IS DIFFERENT THAN THAT SCHEDULED.

26 ALL EQUIPMENT SHALL BE INSTALLED IN STRICT ACCORDANCE WITH THE EQUIPMENT MANUFACTURER'S INSTALLATION INSTRUCTIONS. PROVIDE ALL NECESSARY FITTINGS, TRANSITIONS, VALVES AND OTHER DEVICES AND ACCESSORIES REQUIRED FOR A COMPLETE, WORKABLE INSTALLATION.

27 SEE "PLUMBING FIXTURE SCHEDULE" FOR INDIVIDUAL TRAPS, WASTE, VENT, AND DOMESTIC WATER PIPING FOR INDIVIDUAL FIXTURES.

28 ALL PLUMBING EQUIPMENT SHALL BE LISTED AND LABELED BY AN APPROVED TESTING AGENCY.

29 FIXTURES, EQUIPMENT AND PIPING INSTALLATION SHALL MEET NSF STANDARDS.

PLUMBING SHEET INDEX

P001	PLUMBING COVER SHEET
P101-U	STAGE LEVEL UNDERGROUND PLUMBING
P103-U	ROOF PLUMBING PLAN

STATE OF UTAH
9199431-2202
Michael Nielsen
12.01.2017
LICENSED PROFESSIONAL ENGINEER

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project:

North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

project#: 17.0555
date: 10-5-17

revisions :

title:

PLUMBING COVER SHEET

sheet:

P001

Bid Package #1

SHEET KEYNOTES

SHEET NOTES

PIPE SCHEDULE:
RD: ROOF DRAIN / STORM SEWER
DCW: DOMESTIC COLD WATER
SS: SANITARY SEWER

FIXTURE DRAIN & VENT SIZES:
WATER CLOSET: 4", 2"
LAVATORY: 1-1/2", 1-1/4"
DRINKING FOUNTAIN: 1-1/2", 1-1/4"
WALL MOUNTED SINK: 1-1/2", 1-1/4"
CABINET MOUNTED SINK: 1-1/2", 1-1/4"
FLOOR DRAIN: 2", 2"
FLOOR SINK: 3", 2"
SHOWER DRAIN: 2", 2"
MOP SINK: 3", 2"

NOTE: VENT SYSTEM DESIGN IS INDIVIDUAL FIXTURE VENTS FOR EACH FIXTURE EXCEPT FOR BATHROOM GROUPS. BATHROOM GROUPS WILL BE SERVED BY A WET VENT VIA THE LAVATORY. HORIZONTAL WET VENT SIZE FROM LAVATORY IS 3".

PIPING MATERIALS:
DCW: TYPE L SOFT COPPER WITH WROUGHT FITTINGS.
SS: CAST IRON OR PVC
RD: CAST IRON OR PVC
SEE SPECIFICATIONS FOR FURTHER INFORMATION

DOWNSPOUT NOZZEL
4"ø ORD
4"ø RD
FIRE RISER LOCATION
3"ø RD
3"ø ORD
SCENE SHOP A114
LOADING AREA A113
ADMIN OFFICE A112
CORRIDOR A110
GREEN ROOM A107
STORAGE A109
MECH / ELEC A106
WALL MOUNTED WATER CLOSET
3"ø SS
WALL MOUNTED SINK
MOP SINK
FLOOR CLEANOUT
STAFF RESTROOM A111
WALL MOUNTED LAVATORY
FLOOR DRAIN
CABINET MOUNTED SINK
4"ø SS
2"ø DCW
FLOOR SINK
RESTROOM A108
CUSTODIAL A105
WOMEN'S DRESSING A103
DRINKING FOUNTAIN
RESTROOM A104
RESTROOM A102
4"ø RD
DOWNSPOUT NOZZEL
STORAGE A115
WEST WING A116
STAGE A117
EAST WING A118
MEN'S DRESSING A101

1 STAGE LEVEL UNDERGROUND PLUMBING PLAN
1/4" = 1'-0"

Michael Nielsen
8799431-2202
12.01.2017
STATE OF UTAH
REGISTERED PROFESSIONAL ENGINEER

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project:
North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

project#: 17.0555
date: 10-5-17

revisions :

title:
STAGE LEVEL UNDERGROUND PLUMBING

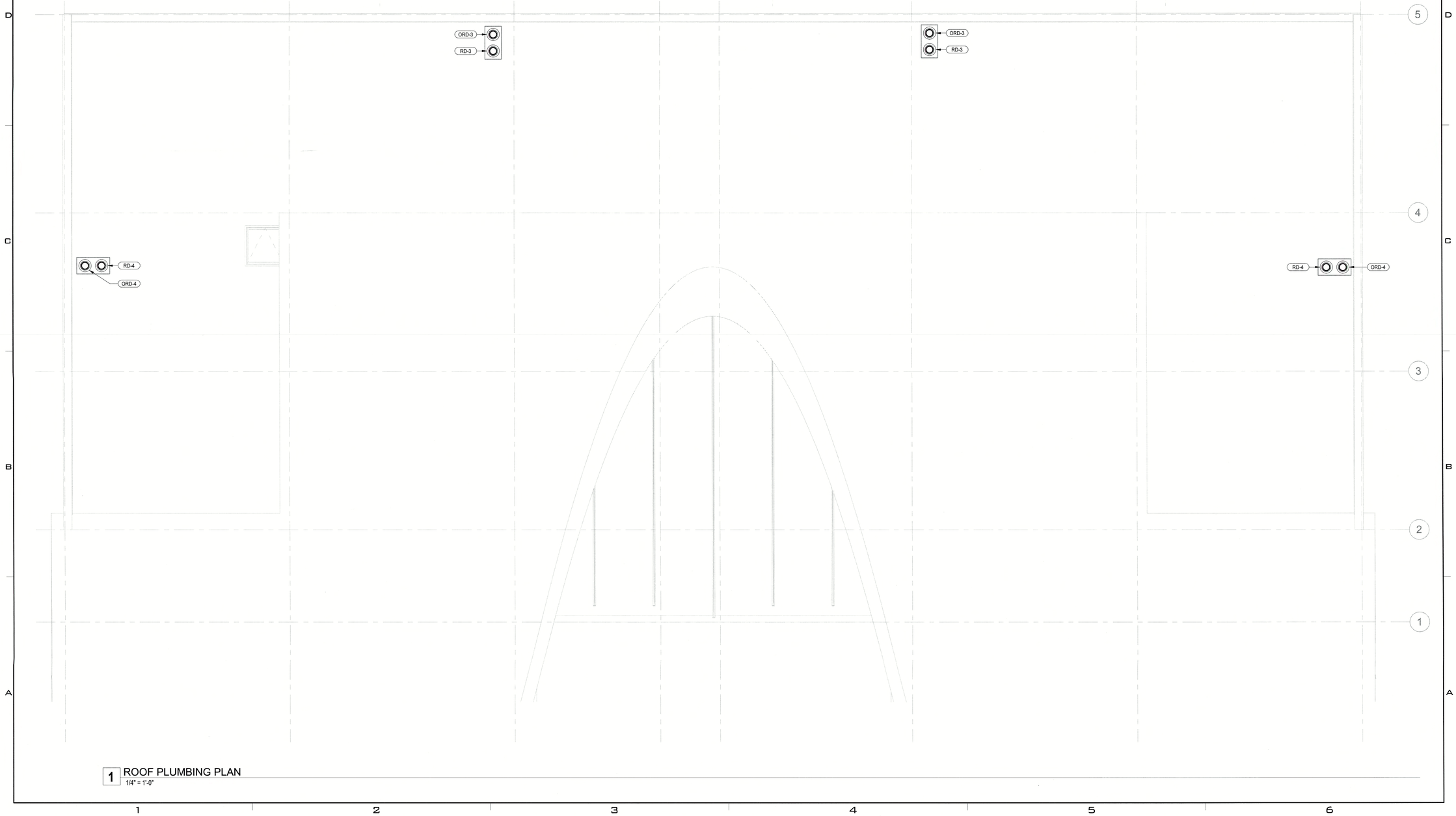
sheet:
P101-U

Bid Package #1

SHEET KEYNOTES

PLUMBING FIXTURE SCHEDULE:
RD-#: CAST IRON BODY, COMBINED FLASHING CLAMP AND GRAVEL STOP, ALUMINUM DOME, SUMP RECEIVER, UNDERDECK CLAMP. '#' INDICATES OUTLET SIZE. BASIS OF DESIGN JR SMITH 1010-AD-R-C.

ORD-#: CAST IRON BODY, COMBINED FLASHING CLAMP AND GRAVEL STOP, 2" WATER DAM, ALUMINUM DOME, SUMP RECEIVER, UNDERDECK CLAMP. '#' INDICATES OUTLET SIZE. BASIS OF DESIGN JR SMITH 1080-AD-R-C.



1 ROOF PLUMBING PLAN
1/4" = 1'-0"

STATE OF UTAH
6769431-2202
Michael
Nielsen
12.01.2017

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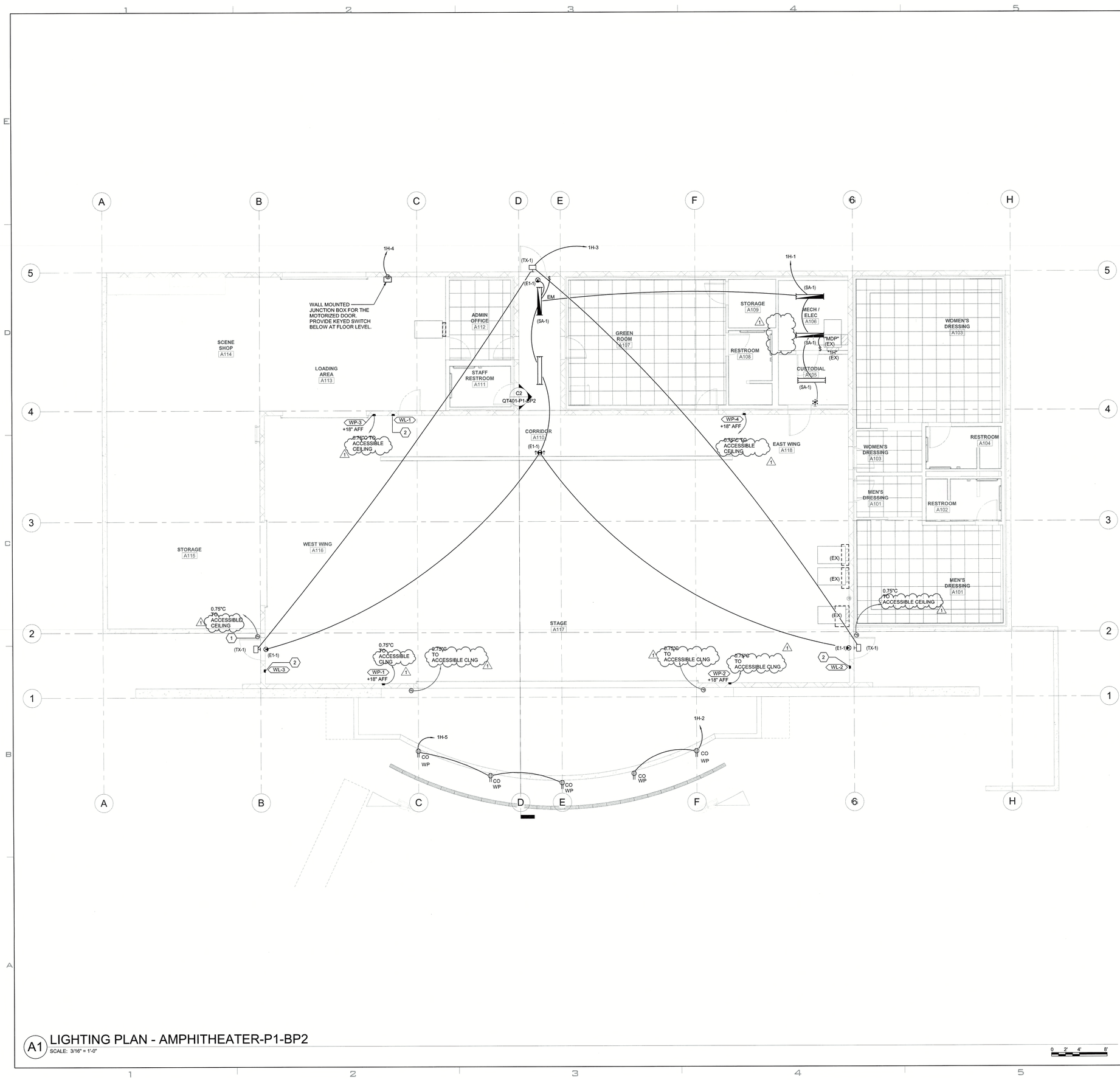
project#: 17-0555
date: 10-5-17

revisions :

title:
ROOF PLUMBING PLAN

sheet:
P103-U

Bid Package #1



A1 LIGHTING PLAN - AMPHITHEATER-P1-BP2

SCALE: 3/16" = 1'-0"

0 2' 4' 8'

GENERAL SHEET NOTES

SHEET KEYNOTES

1 ROUGH IN FOR FUTURE LIGHTING INSTALLATION. COORDINATE EXACT LOCATION WITH ARCHITECT.

2 PROVIDE LIGHT SWITCH FOR WORK LIGHTING. INSTALL IN 3/4"C.

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project:

North Ogden Amphitheater

2375 Fruitland Dr, North Ogden, UT 84414

project#: 17.0555
date: 2017-10-05

revisions :

1	ADDENDUM #1	12-8-17

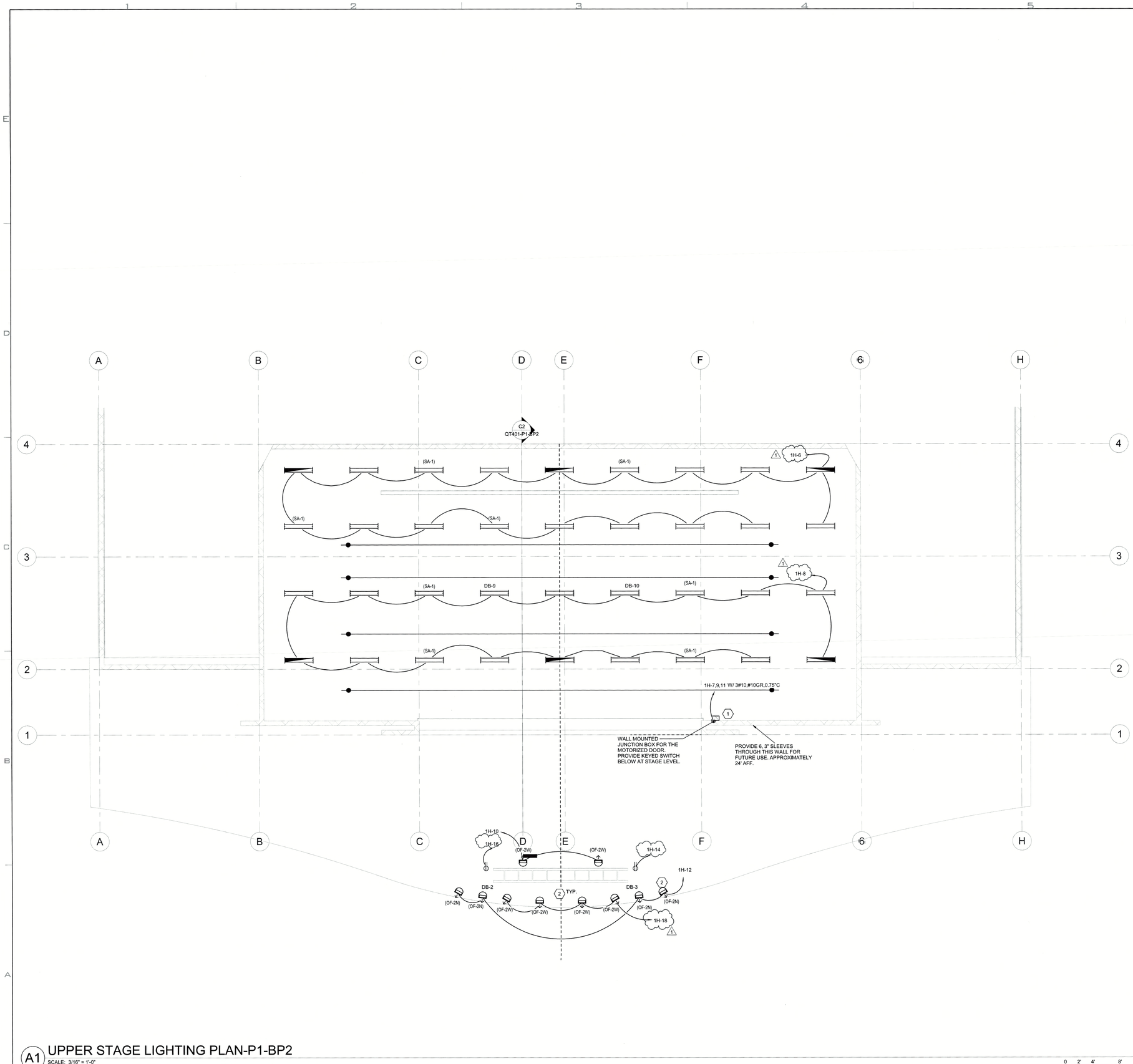
title:

LIGHTING PLAN - AMPHITHEATER

sheet:

EL101-P1-BP2

PHASE 1 BID PACK 2



A1 UPPER STAGE LIGHTING PLAN-P1-BP2
SCALE: 3/16" = 1'-0"

0 2' 4' 8'

GENERAL SHEET NOTES

SHEET KEYNOTES

1 NEW EQUIPMENT P1-BP2

2 OF-XX CANOPY MOUNTED TO ROOF DECK.

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UPPER STAGE LIGHTING PLAN

sheet:
EL102-P1-BP2

PHASE 1 BID PACK 2

INTERIOR LIGHTING FIXTURE SCHEDULE

ABBREVIATIONS

LUMINAIRE OPTIONS

- ARHR - AIR RETURN AND HEAT REJECTION
- DL - DAMP LOCATION
- EQC - EARTHQUAKE CLIPS
- F - FUSING
- HLD - HINGED AND LATCHED DOOR
- HS - HOUSE SIDE SHIELD
- PS - PHOTOCELL SWITCH
- QRS - QUARTZ RESTRIKE
- ST - STATIC
- WG - WIRE GUARD
- WL - WET LOCATION

MOUNTING

- B - BASE
- C - CEILING
- F - FLANGE
- G - GRID
- P - PENDANT
- PL - POLE
- R - RECESSED
- S - SURFACE
- W - WALL

EMERGENCY

- NE - NORMAL AND EMERGENCY CONNECTIONS
- EB - EMERGENCY BATTERY PACK
- ET - EMERGENCY TRANSFER DEVICE

BALLAST

- IS - INSTANT START
- RS - RAPID START
- PS - PROGRAM START, PARALLEL LAMP OPERATION
- PSMH - PULSE START METAL HALLIDE (CWA OR ELECTRONIC)
- PPLF - PROVIDE POWER LINE FILTER
- LVTM - LOW VOLTAGE TRANSFORMER (MAGNETIC)
- LVTE - LOW VOLTAGE TRANSFORMER (ELECTRONIC)

DIMMING BALLAST

- D2 - 2 WIRE DIMMER
- D3 - 3 WIRE DIMMER
- D4 - 4 WIRE DIMMER
- DD - DIGITAL DIMMER
- SDP - STEP DIMMER BALLAST

BALLAST NOTATION

ANSI WATTS = Maximum Wattage at Connected Voltage

BB-#L: BB = Ballast Type, #L = Number of Lamps

Example: PS-2L = (2) Lamp, Program Start Ballast

FINISH

- MW - MATTE WHITE
- BL - BLACK
- SL - SILVER
- GL - GOLD
- CL - CLEAR
- PW - PAINTED WHITE
- EA - EXTRUDED ALUMINUM
- S - STEEL
- GS - GALVANIZED STEEL
- C - CAST
- CBA - COLOR BY ARCHITECT
- SCBA - STANDARD COLOR BY ARCHITECT
- CCA - CUSTOM COLOR BY ARCHITECT
- FS 209D - MEETS FEDERAL STANDARD 209D
- TP - THERMALLY PROTECTED
- FL - FLUSH
- R - REGRESS
- M - MITERED

LENS

- #A - ACRYLIC #THICK
- #OA - ACRYLIC #THICK (OPAL)
- GC - GLASS (CLEAR)
- GO - GLASS (OPAL)
- GF - GLASS (FROSTED)
- SGL - SOFT GLOW LENS
- HPL - HIGH PERFORMANCE LENS
- DO - DROP OPAL
- CGL - CONVEX GLASS LENS
- S - SATIN LENS

OPTICS

- PRF - PERFORATED DIFFUSER
- DT - DETENTION
- BC - BLACK CONE
- BB - BLACK BAFFLE
- WB - WHITE BAFFLE
- VGL - VERTICAL GRAIN LOUVERS
- PFL - LOUVERS IN PLASTIC PROTECTOR
- WW - WALL WASH
- ESL - ELECTROSTATIC SHIELDED LENS

REFLECTOR

- OP - NONE/OPEN
- SP - SPECULAR
- SS - SEMI-SPECULAR
- D - DIFFUSE (WHITE ENAMEL)
- SC - SPECULAR (COLORED)
- PR - PRISMATIC
- FDR - FULL DEPTH REFLECTOR
- DS - DIFFUSE (SEMI SPECULAR) SILVER
- LI - LOW IRIDESCENT
- IR - IRIDESCENT
- SL - SILVER
- GL - GOLD
- CA - CLEAR ALZAK

CIE CLASSIFICATIONS

- DIR - DIRECT LIGHTING
- SD - SEMIDIRECT LIGHTING
- GEN - GENERAL DIFFUSE
- SI - SEMI-INDIRECT LIGHTING
- IND - INDIRECT LIGHTING
- ADJ - ADJUSTABLE

1. VERIFY THE PROPER MOUNTING KITS OR ACCESSORIES TO FACILITATE INSTALLATION AS SHOWN AT EACH LOCATION ON THE DRAWINGS.
2. COMPLY WITH THE "INTERIOR LIGHTING" SECTION OF THE SPECIFICATIONS.
3. REFER TO SPECIFICATIONS FOR IMPORTANT TECHNICAL REQUIREMENTS FOR LIGHTING FIXTURES, BALLASTS, AND LAMPS.
4. ALL FIXTURES SHALL BE APPROVED BY UL OR ANOTHER ACCEPTABLE TESTING LAB FOR THE PURPOSE INTENDED AND WITH THE LAMP AND BALLAST PROPOSED.

NOTES

1. PROVIDE UNIT PRICES AND FIXTURE BRAND SELECTED FOR ADD/DELETE CHANGES FOR EACH FIXTURE TYPES SHOWN WITHIN 48 BUSINESS HOURS OF THE BID DATE. FAILURE TO COMPLY WITH THIS REQUIREMENT MAY DISQUALIFY THE PRODUCTS AND EMPOWER THE ENGINEER TO DETERMINE FAIR VALUE FOR FIXTURE AND INSTALLATION CHANGES, WITHOUT FURTHER INPUT FROM THE CONTRACTOR OR INSTALLER.
2. CONTRACTOR ALLOWANCE PRICES ARE ACCURATE WHEN THIS JOB WAS SPECIFIED, CONTRACTOR AND ELECTRICAL DISTRIBUTOR SHALL VERIFY THIS ALLOWANCE AND REPORT ANY PROBLEMS TO THE ENGINEER BEFORE THE BID. ALLOWANCE PRICE MAY OR MAY NOT INCLUDE LAMP(S) OR FREIGHT AS NOTED, AND DO NOT INCLUDE ANY TAXES.

LIGHTING FIXTUE SCHEDULE - PHASE 1

	LUMINAIRE							LAMP			OPTICS			MANUFACTURER (CATALOG SERIES)					
		NOMINAL SIZE																	
ID	TYPE	LENGTH	WIDTH	MAX DEPTH	DIAMETER/ APERTURE	MOUNTING	OPTIONS	TYPE	LAMP COLOR	QUANTITY	LUMINAIRE LUMENS	OPTIONS	WATTS	OPTION 1	OPTION 2	OPTION 3	OPTION 4	OPTION 5	OPTION 6
(E1-1)	EXIT SIGN WITH BUG-EYES					CS	WIREGUARD	LED	GREEN	0	0		5 W	LITHONIA (LHQM-LED-B-G-HO-ELA WG3)					
(OF-2N)	NARROW SPOT LED FLOOD LIGHT					CS	VISOR	LED	3000K	1	13500		36 W	LITHONIA (DSXF3 LED-6-P1-40K-NSP-MVOLT-THK -DDBXD)					
(OF-2W)	WIDE FLOOR LED FLOOD LIGHT					CS	VISOR	LED	3000K	1	14500		36 W	LITHONIA (DSXF3 LED-6-P1-40K-WFL-MVOLT-THK -DDBXD)					
(SA-1)	INDUSTRIAL STRIP LIGHT, FROSTED LENSED, MATTE BLACK FINISH	48"				CS		LED	4000K	0	3000		25 W	LITHONIA (ZL1N-L48-SMR-3000LM-FST-MVOLT-40K-80CRI-MB)					
(TX-1)	WALL PACK W/ EM BATTERY BACKUP, ONLY ON DURING LOSS OF POWER.	9"	7"			WS		LED	4000K	0	1600		17 W	MULE (MERU-LED-ACEM-CBA)					

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LIGHTING FIXTURE SCHEDULE

sheet:

EL601-P1-BP2

PHASE 1 BID PACK 2

PANEL: "1H"

VOLTS/PHASE/WIRE:	PANEL SIZE & TYPE:	MAIN SIZE AND TYPE:	LOCATION:	CABINET:	NOTES:
120/208V, 3 PH 4 WIRE	22" W x 6" D, BOLT-ON	225 AMPERE MAIN LUGS	MECH / ELEC A106	SURFACE	

ACCESSORIES: PANEL DIRECTORY, IDENTIFICATION, GROUNDING BAR **AIC RATING:** 0

CKT	OCP		LOAD (kVA)				PHASE LOAD							LOAD (kVA)			OCP		CKT
NO	AMP	POLE	LTG	PWR	CO	DESCRIPTION	A		B		C		DESCRIPTION	CO	PWR	LTG	POLE	AMP	NO
1	20	1	0.5	0.0	0.0	LIGHTING ROOM A106, A105, A110	0.5	0.4					LIGHTING: CO STAGE LEFT	0.4	0.0	0.0	1	20	2
3	20	1	0.2	0.0	0.0	LIGHTING			0.2	0.2			OVERHEAD DOOR	0.0	0.2	0.0	1	20	4
5	20	1	0.0	0.0	0.5	LIGHTING: CO STAGE RIGHT					0.5	1.7	LIGHTING: STAGE WORKLIGHTS	0.0	0.0	1.7	1	20	6
7	20	3	0.0	0.2	0.0	OVERHEAD DOOR	0.1	1.7					LIGHTING: STAGE WORKLIGHTS	0.0	0.0	1.7	1	20	8
9	--	--	--	--	--	--			0.1	0.1			LIGHTING: OF-2W STAGE	0.0	0.0	0.1	1	20	10
11	--	--	--	--	--	--					0.1	0.1	LIGHTING: OF-2N HOUSE AISLES	0.0	0.0	0.1	1	20	12
13	20	1	--	--	--	SPARE	0.0	0.0					SPARE	--	--	--	1	20	14
15	20	1	--	--	--	SPARE			0.0	0.0			SPARE	--	--	--	1	20	16
17	20	1	--	--	--	SPARE					0.0	0.0	SPARE	--	--	--	1	20	18
19	20	1	--	--	--	SPARE	0.0	0.0					SPARE	--	--	--	1	20	20
21	20	1	--	--	--	SPARE			0.0	0.0			SPARE	--	--	--	1	20	22
23	20	1	--	--	--	SPARE					0.0	0.0	SPARE	--	--	--	1	20	24
25	20	1	--	--	--	SPARE	0.0	0.0					SPARE	--	--	--	1	20	26
27	20	1	--	--	--	SPARE			0.0	0.0			SPARE	--	--	--	1	20	28
29	20	1	--	--	--	SPARE					0.0	0.0	SPARE	--	--	--	1	20	30
31	20	1	--	--	--	SPARE	0.0	0.0					SPARE	--	--	--	1	20	32
33	20	1	--	--	--	SPARE			0.0	0.0			SPARE	--	--	--	1	20	34
35	20	1	--	--	--	SPARE					0.0	0.0	SPARE	--	--	--	1	20	36
37	20	1	--	--	--	SPARE	0.0	0.0					SPARE	--	--	--	1	20	38
39	20	1	--	--	--	SPARE			0.0	0.0			SPARE	--	--	--	1	20	40
41	20	1	--	--	--	SPARE					0.0	0.0	SPARE	--	--	--	1	20	42
TOTALS:						CONNECTED kVA PER PHASE	**3**		**1**		**2**		CONNECTED TOTAL kVA = **5**						
						CONNECTED AMPS PER PHASE	**24**		**4**		**22**		AVERAGE CONNECTED AMPS PER PHASE = **15**						

NEC DIVERSIFIED LOAD CALCULATIONS

LIGHTING & CONTINUOUS LOADS: **4.3 kVA @ 125% = 5.3 kVA** - 100% CONNECTED LOAD PLUS 25%

RECEPTACLES: **0.9 kVA @ 100% = 0.9 kVA** - FIRST 10kVA @ 100%, REMAINDER @ 50%

ALL OTHER LOADS @ 100% : **0.4 kVA** - MOTOR TOTALS INCLUDED IN ALL OTHER LOADS WITH LARGEST MOTOR CALCULATED @ 125% PER NEC

DIVERSIFIED TOTAL kVA = **7**

AVERAGE AMPS PER PHASE = **18**

(PANEL IS EXISTING. NEW 20/3 BREAKER WILL BE REQUIRED AS EXISTING BREAKERS ARE 20/1)

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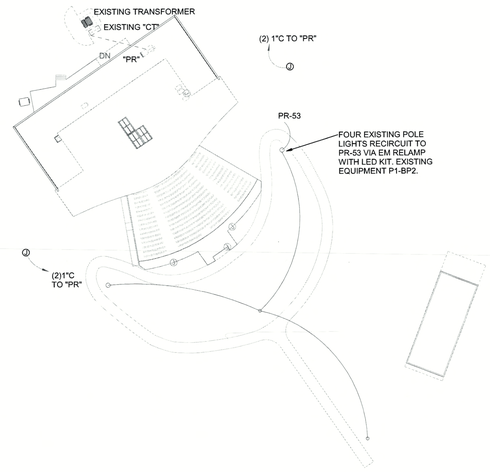
title:

PANEL SCHEDULES

sheet:

EP604

PHASE 1 BID PACK 2



A1 ELECTRICAL SITE PLAN P1-BP2
SCALE: 1" = 40'-0"

0 2' 4' 8'

LICENSED PROFESSIONAL ENGINEER
No. 5148728
DAVID G. HINCKLEY
STATE OF UTAH

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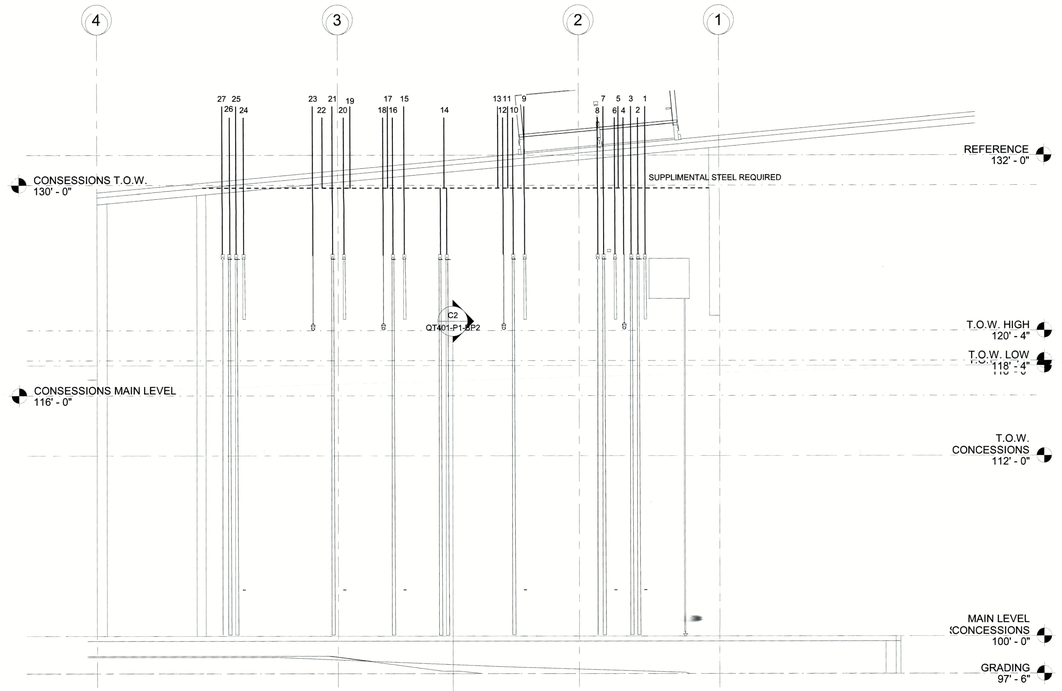
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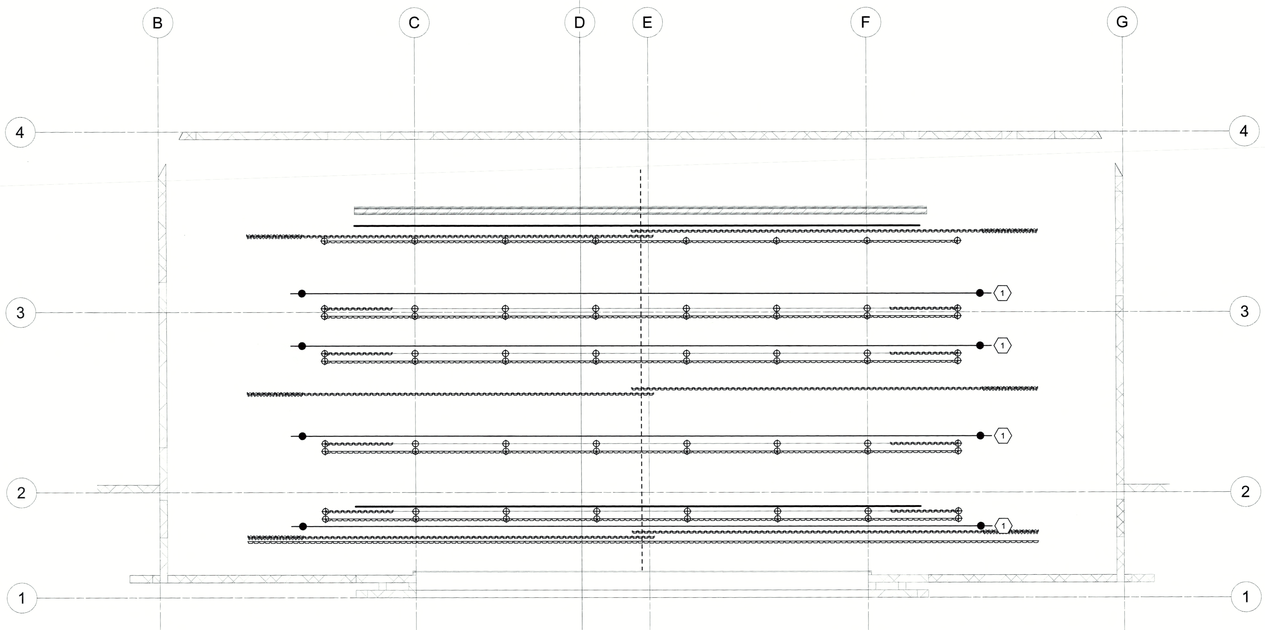
title:
ELECTRICAL SITE PLAN

sheet:
ES101-P1-BP2

BID PACKAGE 1



C2 RIGGING SECTION
SCALE: 1/4" = 1'-0"



GENERAL SHEET NOTES

SHEET KEYNOTES

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STAGE RIGGING PLAN AND SECTION

RIGGING SCHEDULE

LINE #	LOCATION	DESCRIPTION	TRACK (SEE NOTE)	PIPE LENGTH	SIZE (W x H)	MATERIAL	FABRIC WEIGHT	COLOR	FULL-NESS	OTHER	# OF LINES	BASE	ALTERNATE
E		HOUSE SIDE OF PROC WALL											X
F		SPEAKERS											X
G		CENTER TRUSS	H&H 400	20'								X	
H		SPACE											
I		SIDE TRUSSES		10' EACH									X
J													
0	0'-0"	STAGE SIDE OF PROC WALL											
1	3'-0"	GRAND TEASER		44'-0"	44'-0"W x 4'-0"H	IFR VELOUR	25 oz	TBD	100%	LINED	8		X
2	3'-8"	GRAND DRAPE	H&H 400	70'-0"	2 @ 22'-0"W x 24'-0"H	IFR VELOUR	25 oz	TBD	100%	LINED	8		X
3		SPACE											
4	4'-4"	FIRST ELECTRIC		60'-0"	64'-0" LONG						9	X	
5		SPACE											
6	5'-0"	FIRST TEASER		45'-0"	45'-0"W x 5'-0"H	IFR VELOUR	15 oz	BLACK	50%		8		X
7	5'-8"	FIRST LEGS	2 @ 18'-0"	60'-0"	2 @ 10'-0"W x 22'-0"H	IFR VELOUR	15 ox	BLACK	50%		8		X
8	6'-0"	PROJECTION SCREEN		30'-0"	28' x 15' x 7' DROP	ELECTRICALLY OPERATED ROLL UP, SWITCH AT SMP					N/A		X
9	11'-0"	SECOND TEASER		45'-0"	45'-0"W x 6'-0"H	IFR VELOUR	15 OZ	BLACK	50%		8		X
10	11'-8"	SECOND LEGS	2 @ 18'-0"	60'-0"	2 @ 10'-0"W x 22'-0"H	IFR VELOUR	15 oz	BLACK	50%		8		X
11		SPACE											
12	12'-4"	SECOND ELECTRIC		60'-0"	60'-0" LONG						9	X	
13		SPACE											
14	16'-4"	MID STAGE CUT OFF	H&H 400	70'-0"	2 @ 33'-0"W x 22'-0"H	IFR VELOUR	15 OZ	BLACK	75%	LINED	8		X
15	19'-0"	THIRD TEASER		45'-0"	45'-0"W x 6'-0"H	IFR VELOUR	15 OZ	BLACK	50%		8		X
16	19'-8"	THIRD LEGS	2 @ 18'-0"	60'-0"	2 @ 10'-0"W x 22'-0"H	IFR VELOUR	15 OZ	BLACK	50%		8		X
17		SPACE											
18	20'-4"	THIRD ELECTRIC		60'-0"	60'-0" LONG						9	X	
19		SPACE											
20	23'-0"	FOURTH TEASER		45'-0"	45'-0"W x 6'-0"H	IFR VELOUR	15 OZ	BLACK	50%		8		X
21	23'-8"	FOURTH LEGS	2 @ 18'-0"	60'-0"	2 @ 10'-0"W x 22'-0"H	IFR VELOUR	15 OZ	BLACK	50%		8		X
22		SPACE											
23	25'-0"	FOURTH ELECTRIC		60'-0"	60'-0" LONG						9	X	
24		SPACE											
25	29'-8"	FIFTH TEASER		45'-0"	45'-0"W x 7'-0"H	IFR VELOUR	15 OZ	BLACK			8		X
26	30'-4"	UPSTAGE TRAVELLER	H&H 400	70'-0"	2 @ 33'-0"W x 22'-0"H	IFR VELOUR	15 OZ	BLACK	75%		8		
27	31'-0"	CYC		60'-0"	60'-0"W x 22'-0"H	MUSLIN		NATURAL	0%	SEAMLESS	8		X

NOTE: ALL TRACKS NET LENGTH: ADD FOR OVERLAP, PULLEYS, ETC.
ALL DEADHUNG LINES TO BE SECURED TO STRUCTURE WITH SUITABLE BEAM CLAMP.
CONSULT DIVISION 1 AND BID FORM FOR INFORMATION CONCERNING BASE BID AND ALTERNATE BID/UNIT PRICE ALLOCATION OF THIS EQUIPMENT.

Notes to Structural Engineer:
Weights are generally uniformly distributed over the stage.
This weight will appear at the head block beam with both vertial and horizontal forces.
The 8 liftlines are 8' o/c centered from the center line of the procenium.
I have asked for an arbor pit to allow full travel of the arbors. See pasted detail.

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THEATER EQUIPMENT SCHEDULES

sheet:

QT601-P1-BP2

PHASE 1 BID PACK 2